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MECHANICAL

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FIRE DEPARTMENT

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VICINITY MAP

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CONSTRUCTION OF VSU POWER PLANT BUILDING LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A

ENGR. MARIO LILIO P. VALENZONA DR. DANIEL LESLIE S. TAN DIRECTOR, PPO VP OF ADMINISTRATIVE AND FINANCE

CONFORMED / APPROVED BY:

DR. EDGARDO E. TULIN VSU PRESIDENT

APPROVED BY:

PERSPECTIVE VICINITY MAP SITE DEVELOPMENT PLAN

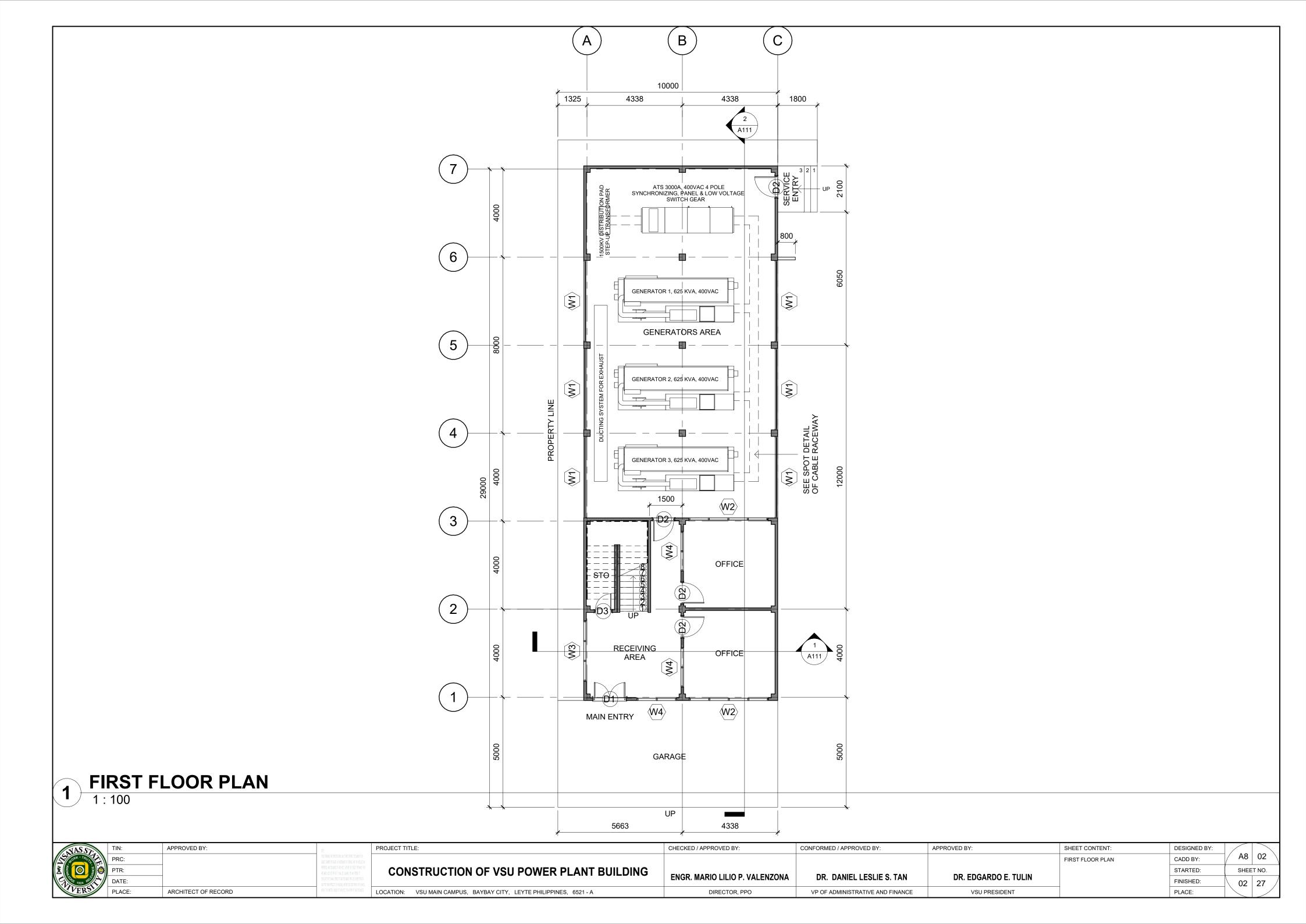
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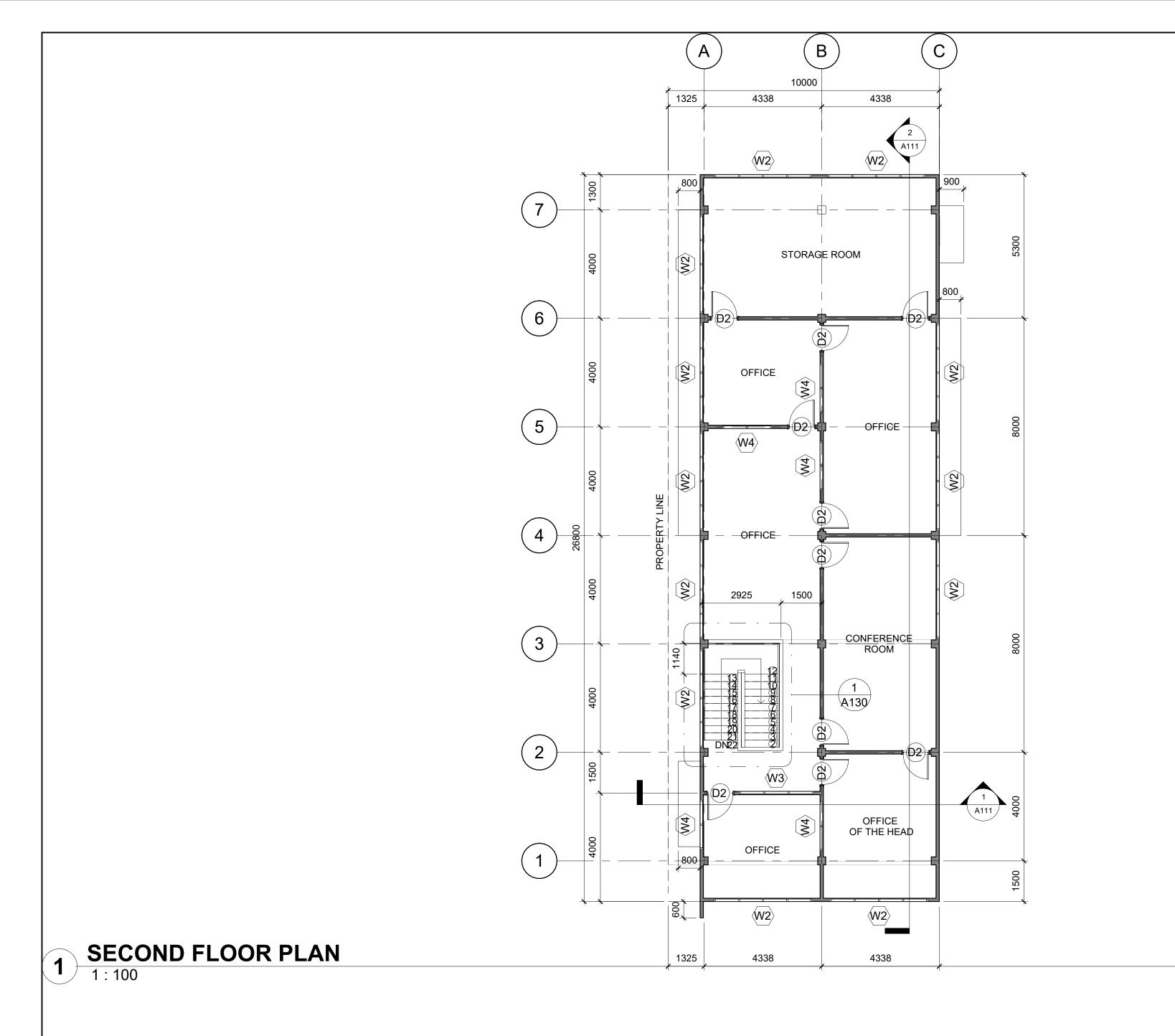
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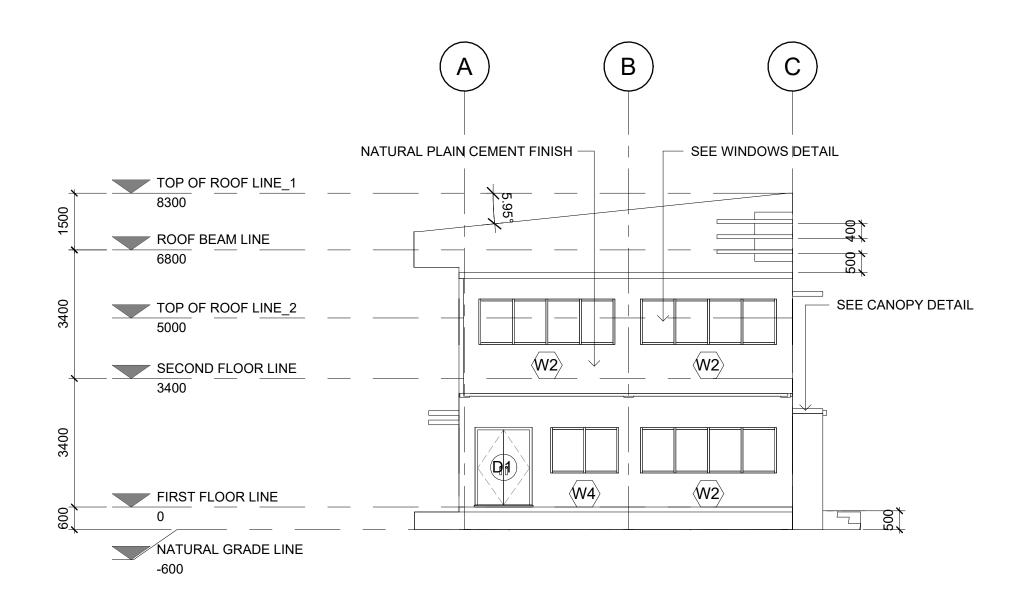
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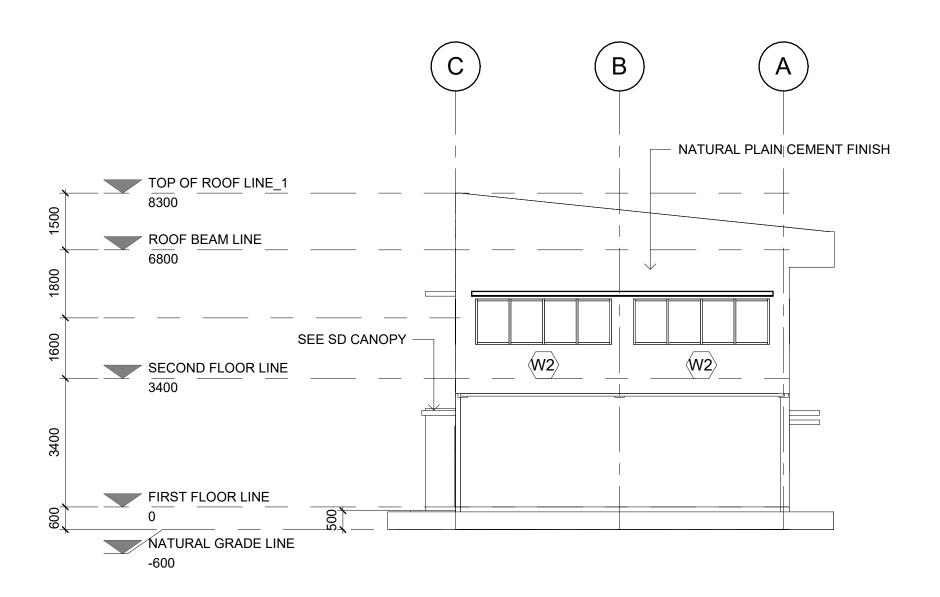


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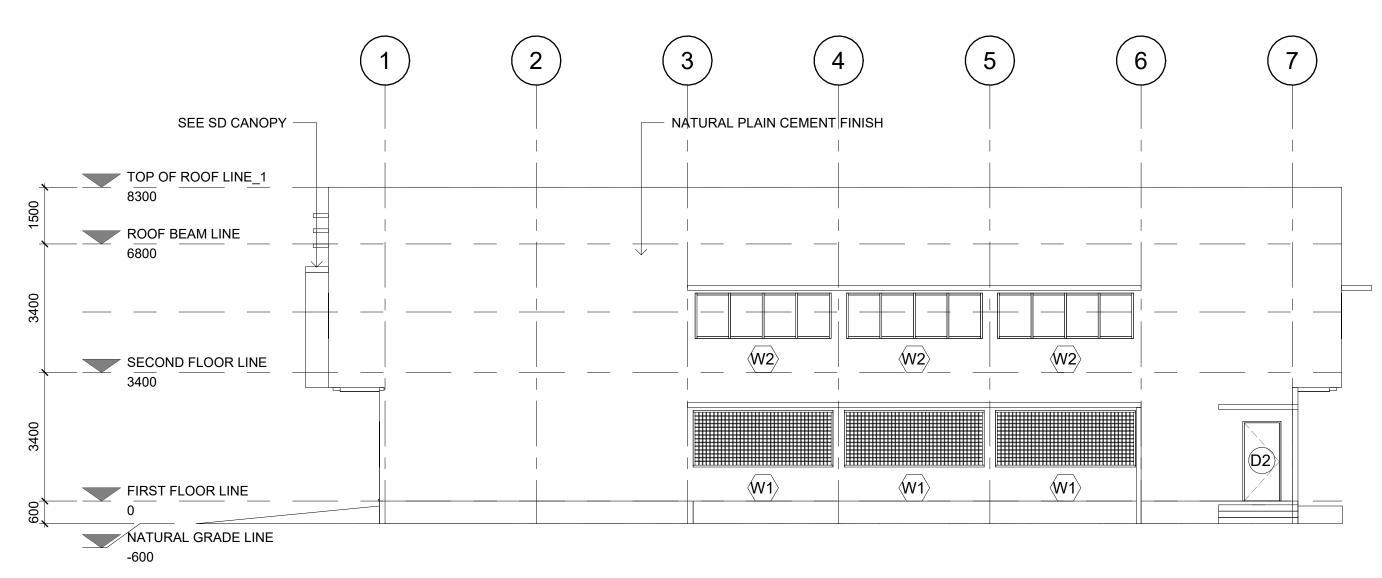
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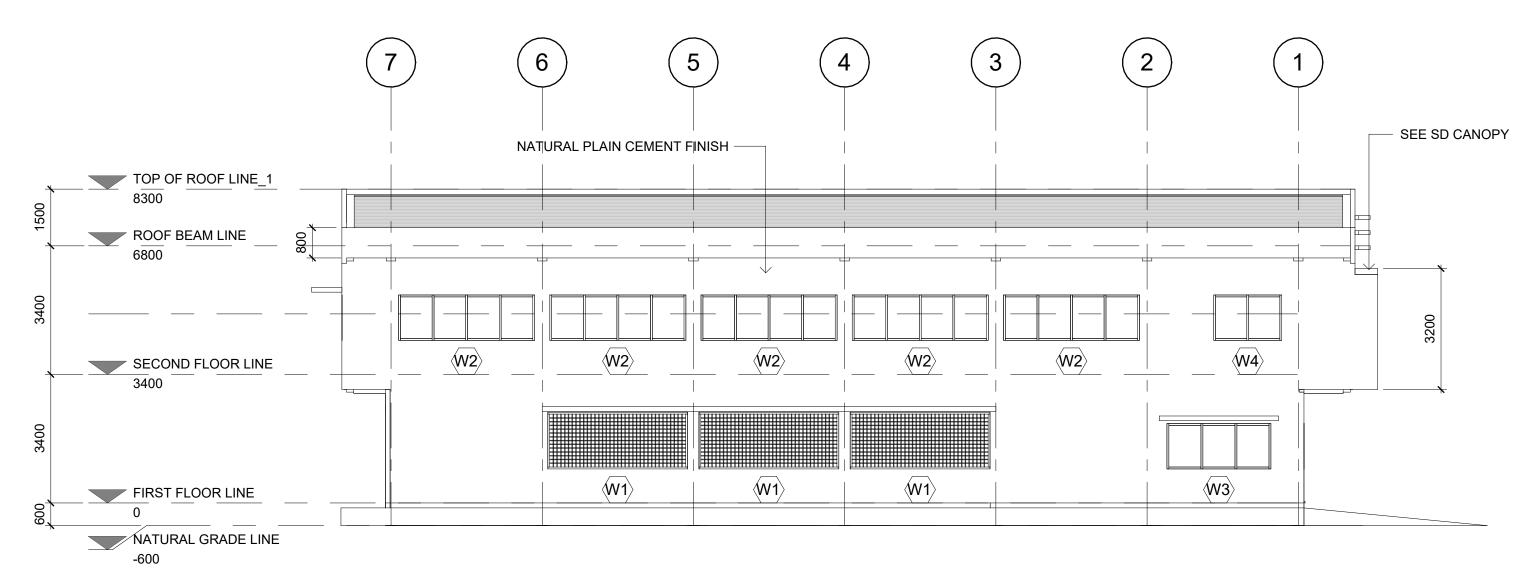


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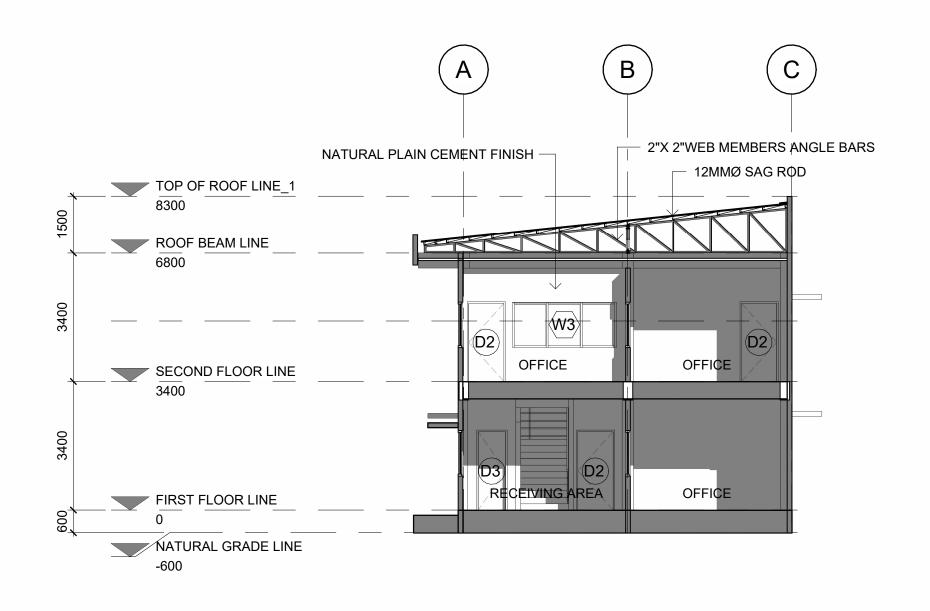


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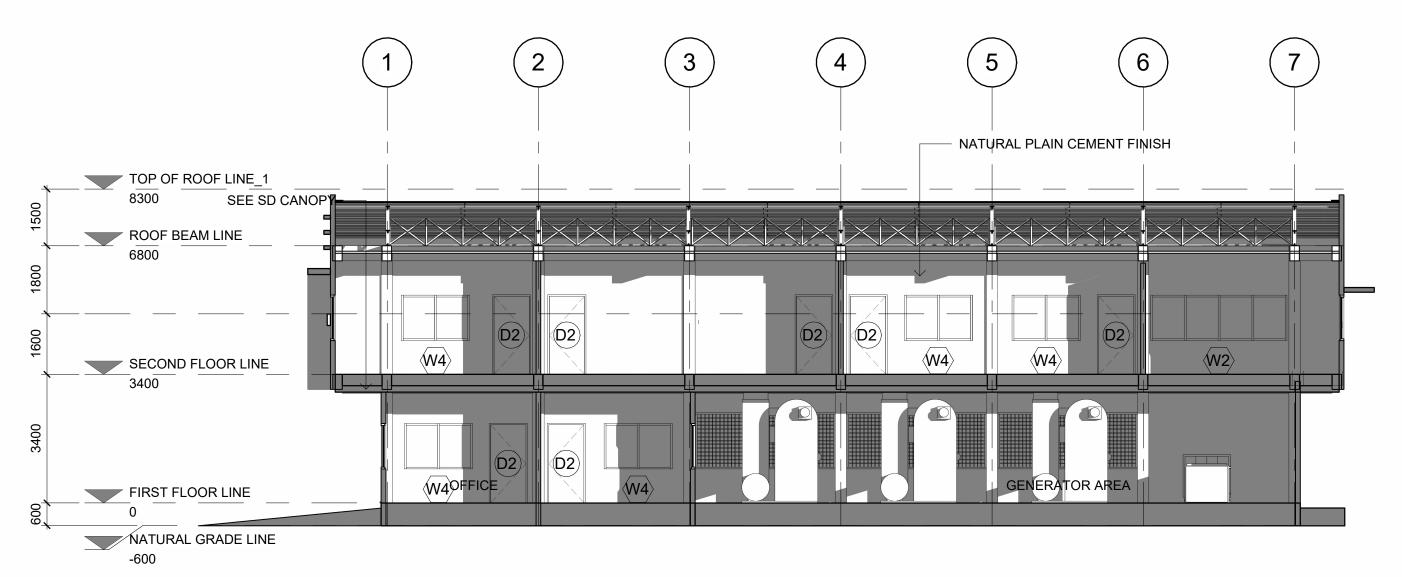
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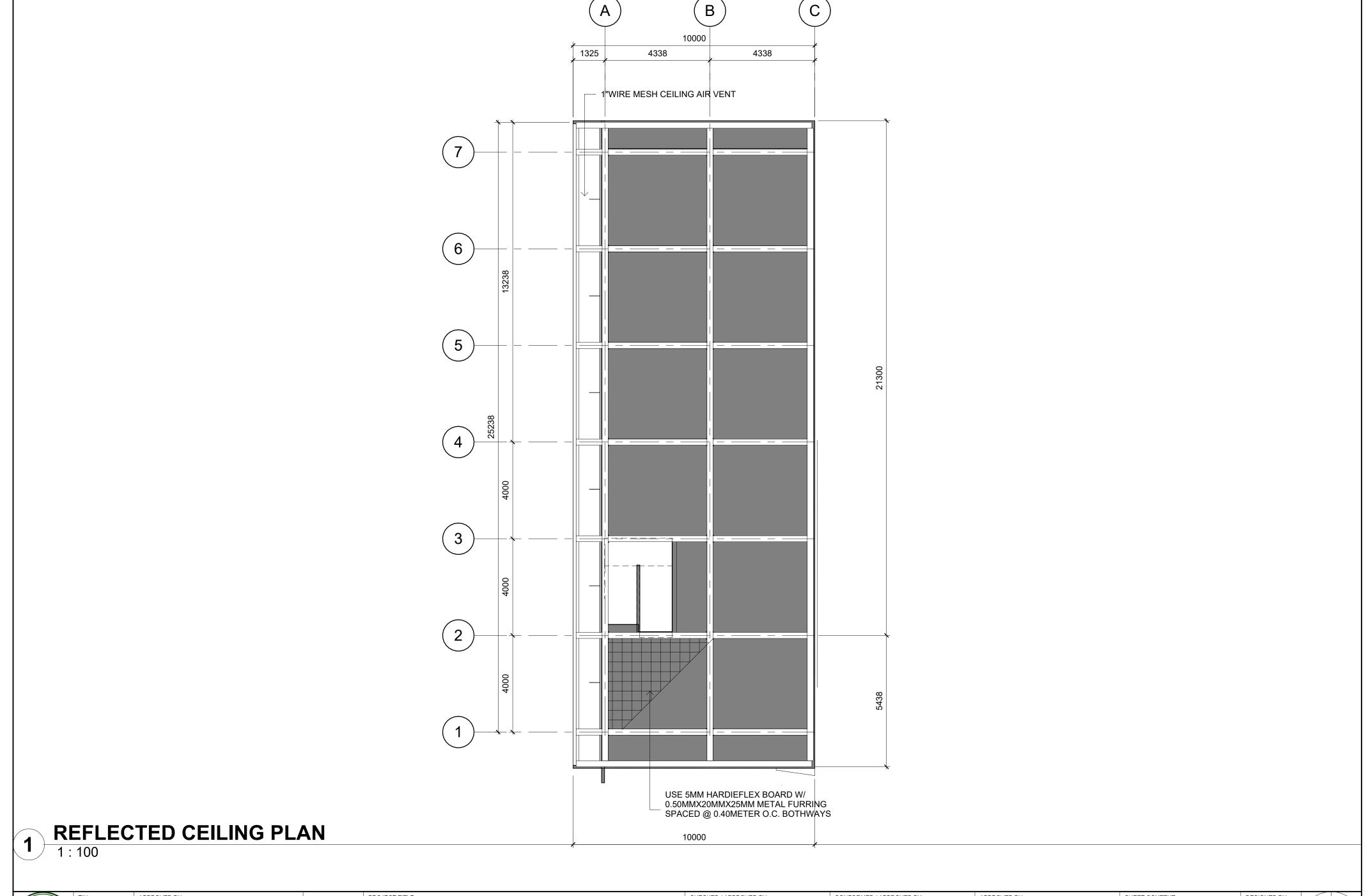
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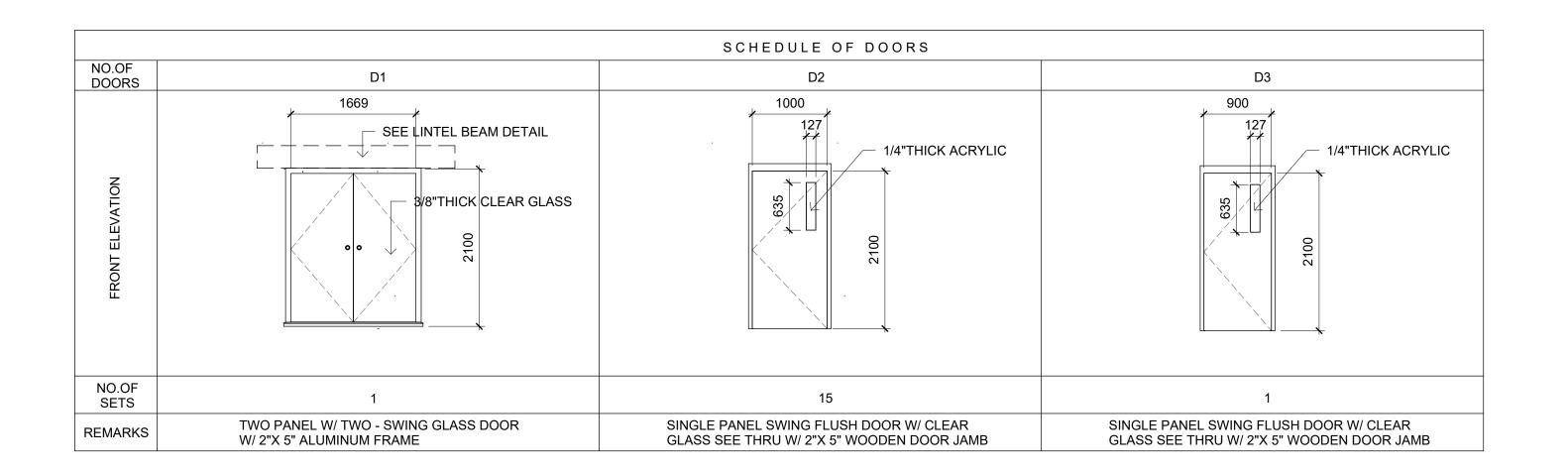


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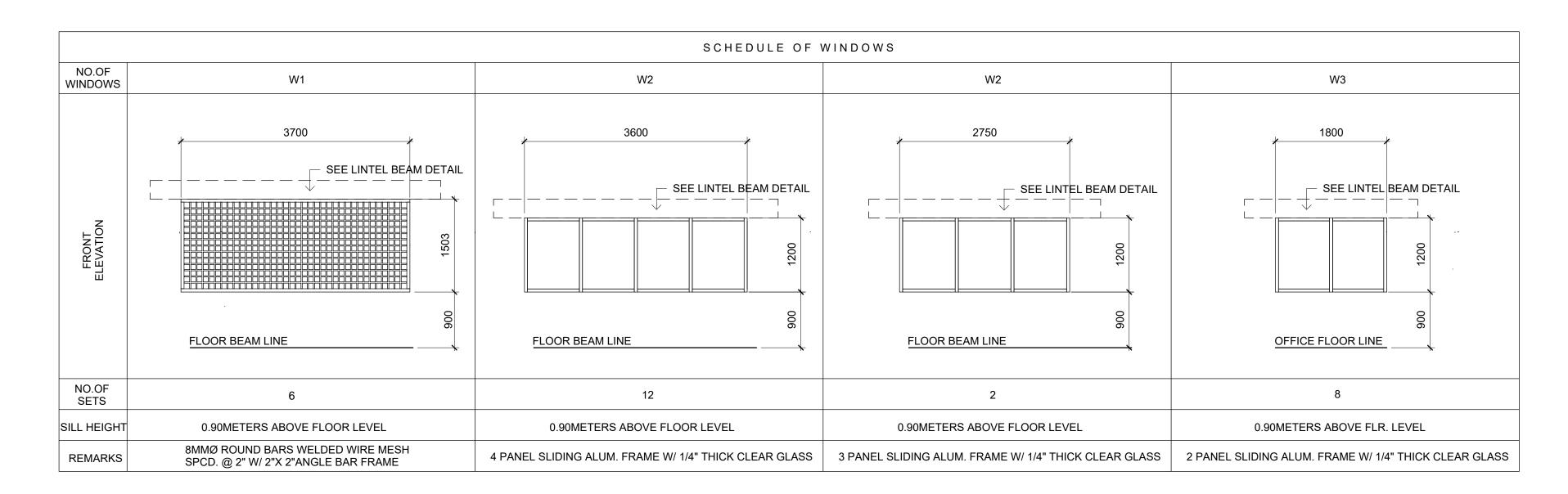


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DOOR LEGEND

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WINDOW LEGEND

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GENERAL NOTES

1 WORKING DRAWINGS

THIS "GENERAL NOTES FOR STRUCTURAL WORKS" SHALL FORM A PART OF THE STRUCTURAL WORKING DRAWING.

IN THE INTERPRETATION OF THIS DRAWING, INDICATED DIMENSIONS SHALL GOVERN AND DISTANCE OR SIZES SHALL NOT BE SCALED FOR CONSTRUCTION PURPOSES.

IN CASES OF CONFLICT IN DETAILS AND DIMENSIONS BETWEEN THE ARCHITECTURAL AND STRUCTURAL DRAWINGS, REFER TO THE ARCHITECT OR HIS AUTHORIZED REPRESENTATIVE FOR FINAL DECISION.

IN CASES OF CONFLICT BETWEEN THE DRAWINGS, GENERAL NOTES AND SPECIFICATIONS, THE WORKING DRAWING SHALL GOVERN

IN CASES OF CONFLICT BETWEEN THIS GENERAL NOTES AND SPECIFICATIONS, THE GENERAL NOTES SHALL GOVERN.

DESIGN LOADS_

DEAD LOADS

THE DESIGN DEAD LOAD UNLESS OTHERWISE SPECIFIED IN THE STRUCTURAL PLANS ARE AS FOLLOWS:

FLOOR MORTAR FINISH = 10 psf OF FLOOR AREA FLOOR ARCHITECTURAL FINISH = 10 psf OF FLOOR AREA CEILING AND UTILITIES = 5 psf OF FLOOR AREA DRYWALL PARTITIONS = 20 psf OF FLOOR AREA WATERPROOFING AND = 25 psf OF FLOOR AREA CONCRETE COVER

THE ARCHITECTURAL AND BUILDING CONTRACTOR SHALL GET THE APPROVAL OF THE STRUCTURAL ENGINEER OF ANY LOADING DIFFERENT FROM OR EXCEEDING THE DESIGN LOADS.

2 LIVE LOADS

THE DESIGN LIVE LOAD UNLESS OTHERWISE SPECIFIED IN THE PLANS ARE AS FOLLOWS :

TYPICAL FLOOR = 100 psf MECHANICAL & ELECTRICAL = 250 psf ROOF DECK = 100 psf ROOF DECK (NON-ACCESSIBLE) = 30 psf

THESE DESIGN LIVE LOADS SHALL NEVER BE EXCEEDED AT ANY TIME DURING THE LIFE OF THE STRUCTURE WITHOUT THE WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.

IT SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE BUILDING TO GET THE APPROVAL OF THE STRUCTURAL ENGINEER OF ANY LIVE LOAD WHICH WILL BE IMPOSED ON AN AREA OF THE BUILDING DIFFERENT FROM AND OR EXCEEDING THE DESIGN LIVE LOADS SPECIFIED HEREIN.

NO PORTION OF THE BUILDING SHALL BE USED AS TEMPORARY STORAGE OF CONSTRUCTION MATERIALS IN EXCESS OF THE DESIGN LIVE LOADS WIHOUT THE CONSENT OF THE STRUCTURAL ENGINEER.

3 EARTHQUAKE LOADS

THE DESIGN EATHQUAKE LOADS ARE AS PER NATIONAL STRUCTURAL CODE OF THE PHILIPPINES 2001 EDITION

WIND LOADS

THE DESIGN WIND LOADS ARE AS PER NATIONAL STRUCTURAL CODE OF THE PHILIPPINES 2001 EDITION

.5 EQUIPMENT LOADING EQUIPMENT NOT INDICATED IN THE PLANS SHALL NOT BE INSTALLED WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.

ENGINEER.
THE MANUFACTURER SHALL SUBMIT EQUIPMENT DATA SPECIFYING THE WEIGHT, AND ITS REACTION AT THE BASE, AND ITS VIBRATION CHARACTERISTICS.

REINFORCED CONCRETE CONSTRUCTION_

- 1 CEMENT SHALL CONFORM TO PORTLAND CEMENT ASTM C150.
- .2 CONCRETE AGGREGATE SHALL CONFORM TO ASTM C33 EXCEPT THAT AGGREGATES FAILING TO MEET THOSE SPECIFICATION BUT WHICH HAVE PRODUCED CONCRETE OF ADEQUATE STRENGTH AND DURABILITY MAYBE USED SUBJECT TO THE APPROVAL OF THE FNGINFER
- .3 WATER USED IN MIXING CONCRETE SHALL BE CLEAN AND FREE FROM INJURIOUS AMOUNT OF OILS, ACIDS, ALKALIS, SALTS, ORGANIC MATERIALS OR OTHER SUBSTANCES DELETERIOUS TO CONCRETE AND STEEL.
- 4 REINFORCING BARS SHALL CONFORM TO ASTM A615.
- .5 ADMIXTURES TO BE USED IN CONCRETE SHALL BE SUBJECT TO PRIOR APPROVAL BY THE STRUCTURAL ENGINEER.
- 6 CEMENT AND AGGREGATES SHALL BE STORED IN SUCH A MANNER AS TO PREVENT THEIR DETERIORATION OR THE INTRU—SION OF FOREIGN MATTER.

- 7 CONCRETE CYLINDER SAMPLES FOR STRENGTH TESTS OF EACH CLASS OR CONCRETE SHALL BE TAKEN NOT LESS THAN TWICE A DAY OR NOR LESS THAN ONCE FOR EACH 50 CU. m. OF CONCRETE PLACE.
- 8 THE CYLINDER SAMPLES FOR STRENGTH TESTS SHALL BE TAKEN CURED AND TESTED IN ACCORDANCE WITH ASTM C172, ASTM C31. ASTM C39.
- 9 ACCEPTANCE OF CONCRETE. CONCRETE POURED WILL BE CONSIDERED SATISFACTORY IF THE AVERAGES OF ALL SETS OF THREE CONSECUTIVE STRENGTH TEST RESULTS EQUAL OR EXCEED THE REQUIRED fc' AND NO INDIVIDUAL STRENGTH TEST RESULTS FALL BELOW THE REQUIRED fc' BY MORE THAN 500
- 10 CORE TESTS AND LOAD TESTS. IF INDIVIDUAL TESTS OF LABORATORY CURED CYLINDER SAMPLES PRODUCED STRENGTH MORE THAN 500 psi BELOW fc' CORE TESTS MAY BE RESORTED SUBJECT TO THE APPROVAL BY THE STRUCTURAL ENGINEER.
- 11 MIXING OF CONCRETE. ALL CONCRETE SHALL BE MIXED UNTIL THERE IS UNIFORM DISTRIBUTION OF THE MATERIALS AND SHALL BE DISCHARGED COMPLETELY BEFORE THE MIXER IS RECHARGED
- 12 CONVEYING OF CONCRETE. CONCRETE SHALL BE CONVEYED FROM THE MIXER TO THE PLACE OF FINAL DEPOSIT BY METHODS WHICH WILL PREVENT THE SEPARATION OR LOSS OF MATERIALS.
- 13 DEPOSITING OF CONCRETE. CONCRETE SHALL BE DEPOSITED AS NEARLY AS PRACTICABLE IN ITS FINAL POSITION TO AVOID SEGREGATION DUE TO REHANDLING OR FLOWING.
- 14 CURING. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR AT LEAST 7 DAYS AFTER PLACING. WET BURLAP MAYBE LAYED OVER THE SLAB CONSTANTLY APPLIED WITH WATER

CONCRETE MIXES_

UNLESS OTHERWISE INDICATED IN THE DRAWINGS, THE MINIMUM 28-DAYS CYLINDER COMPRESSIVE STRENGTH SHALL BE AS

A. FOUNDATION, CONCRETE WALLS, COLUMNS, UNDERGROUND TANKS, SUSPENDED BEAMS AND SLABS

fc' = 4,000 psi

SLAB ON GRADE fc' = 3,000 psi

COTINGS

- 1 THE ALLOWABLE SOIL BEARING PRESSURE IS 6,000 PSF
- 2 EXCAVATION FOR FOOTINGS SHALL BE CARRIED TO A DEPTH AS SPECIFIED IN THE PLANS
- 3 EXISTING UNDERGROUND PIPES, TUNNELS, ETC. SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR EVALUATION
- ALL EARTH FILL SUPPORTING BASEMENT SLABS FOR FLOORING, PARKING, SIDEWALK, ETC. SHALL BE COMPACTED TO 95% PROCTOR UNLESS OTHERWISE SPECIFIED BY THE STRUCTURAL ENGINEER.

REINFORCED CONCRETE BEAMS

- UNLESS OTHERWISE NOTED IN THE PLANS OR SPECIFICATIONS CAMBER ALL RC BEAMS AT LEAST 6 mm FOR EVERY 4.5 m. OF SPAN EXCEPT CANTILEVERS WHICH SHALL BE 18 mm FOR EVERY 3.0 m. SPAN.
- 2 IF THERE ARE TWO OR MORE LAYERS OF REINFORCING BARS, USE 250 SEPARATORS SPACED AT 1200 mm O.C.
- IF BEAMS REINFORCING BARS ENDS IN A WALL, THE CLEAR DISTANCE FROM THE BAR TO THE FARTHER FACE OF THE WALL SHALL BE NOT LESS THAN 5 DIAMETERS. THE REINFORCING BARS SHALL TERMINATE ON A STANDARD OF 90° HOOK.
- SPLICES FOR TOP BARS SHALL BE LOCATED AT MID—SPAN AND BOTTOM BARS SHALL BE SPLICED AT THE DISTANCE OF TWICE THE MEMBER DEPTH FROM THE FACE OF THE COLUMN.

 LENGTH OF SPLICE SHALL BE HELD TO PROVIDE A MINIMUM OF TWO STIRRUPS TIES AT SPLICE. SPLICES INDICATE MAYBE STAGGERED A DISTANCE OF AT LEAST 40 BARS DIAMETER A MAXIMUM OF 50% OF TOP OR BOTTOM BAR MAY BE SPLICED AT ANY ONE LOCATION.

REINFORCED CONCRETE SLABS_

- 1 UNLESS OTHERWISE NOTED IN PLANS OR SPECIFICATIONS, CAMBER ALL R.C. SLABS 3 mm PER 3.0 m. OF SHORTER SPAN.
- 2 IF SLABS ARE REINFORCED BOTH WAYS, THE SHORTER SPAN REBAR SHALL BE THE BOTTOM BARS.
 3 CONCRETE COVERING SHALL BE 19 mm FOR TOP AND
- 4 UNLESS OTHERWISE SPECIFIED BY THE ENGINEER BAR CHAIRS SHALL BE PROVIDED AT LEAST 0.90 m. EACH WAY TO SUPPORT TOP AND BOTTOM SLAB SEPARATELY.

REINFORCED CONCRETE COLUMNS_

- CONSTRUCTION JOINTS IN COLUMNS SHALL BE LOCATED AT MIDHEIGHT OF COLUMN (BETWEEN FLOOR LINE.)
- 2 ALL TIES SHALL BE FASTENED TO COLUMN VERTICAL REINFORCE— MENT BY MEANS OF WIRES AT ALL INTERSECTION POINTS OF TIES AND COLUMN REBARS.
- 3 NOT MORE THAN ONE—THIRD (1/3) OF THE TOTAL NUMBER OF BARS SHALL BE SPLICED AT THE SAME LEVEL. THE LAP SPLICE SHALL BE 1.2 Ld. WHERE Ld IS THE DEVELOPMENT LENGTH AS TABULATED IN THE STANDARD DETAILS. SPLICES SHALL BE STAGGERED A DISTANCE OF AT LEAST Ld.
- 4 CONFINEMENT TIES SHALL BE PROVIDED ON ALL COLUMNS AT BEAM COLUMN INTERSECTIONS AS SHOWN IN STANDARD DETAILS.

REINFORCED CONCRETE WALLS

- UNLESS OTHERWISE INDICATED IN THE PLANS THE R.C. WALL HORIZONTAL BARS SHALL ENCLOSE THE VERTICAL BARS.
- CONCRETE CLEARANCES FOR REINFORCING BARS (SEE ITEM 12) FOR 100 mm WALL OR LESS REINFORCING BARS SHALL BE
- CARRY VERTICAL BARS 0.60 m. ABOVE FLOOR LEVEL TO PROVIDE FOR SPLICES WHEN NECESSARY. ELSEWHERE STOP AT 50 mm BELOW THE TOP OF THE SLAB. THE BAR SHALL TERMINATE ON STANDARD 90° DEGREE HOOK.
- 4 HORIZONTAL AND VERTICAL BARS CONFORMING TO ASTM A615 SHALL BE SPLICED BY LAPPING AND WIRED WITH NO.16 G.I. WIRE AND CONFORMING SECTION 1 PROVIDED THAT SPLICES IN ADJACENT BARS ARE STAGGERED AT LEAST 1.50 m. O.C. NOT MORE THAN ONE—HALF (1/2) OF THE TOTAL NUMBER OF BARS SHALL BE SPLICED AT THE SAME LINE.
- 5 ALL OPENINGS ON WALL OR SLABS LESS THAN 200 mm. THICK SHALL BE REINFORCED AS SHOWN IN THE STANDARD DETAILS.

CHR WALLS

- .1 UNLESS OTHERWISE INDICATED THE VERTICAL AND HORIZONTAL REINFORCEMENTS FOR C.H.B. SHALL BE 10 mm. AT 600 O.C. FOR 0.15 m. THICK BLOCKS. MINIMUM LAP SPLICE SHALL BE 300 mm. (REFER TO ARCHITECTURAL FLOOR PLAN FOR LOCATION OF C.H.B. WALLS).
- 2 LINTEL BEAMS TO BE USED SHALL BE (T X 0.40 m.)
 REINFORCED BY 4-16 mm WITH 10 mm AT 250 TIES WHERE
 "T" IS THE CHB WALL THICKNESS. (UNLESS OTHERWISE SPECIFIED ON DETAILS).
- .3 LINTEL BEAMS SHALL BE PROVIDED AT TOP OF CHB WALL OPENINGS IT SHALL BE EXTENDED AT LEAST 0.20 m. BEYOND THE OPENINGS.
- .4 FOR HIGH WALLS, LINTEL BEAMS INTERVAL SHALL BE PROVIDED AT 3.0 m. 3.0 m. VERTICAL
- 5.0 m. VERTICAL

 .5 FOR LONG WALLS, LINTEL BEAMS ACTING AS COLUMN SHALL BE
- PROVIDED EVERY 6 METERS

 6 WHERE CHB WALLS ADJOIN R.C. COLUMNS AND BEAMS, PROVIDE DOWELS ON R.C. COLUMN AND BEAMS PRIOR TO POURING TO
- MATCH CHB WALL REINFORCEMENT.

 7 WHERE THE TOP OF A CHB WALL ADJOIN A BEAM OR SLAB, REBARS AS REQUIRED IN 10.6 SHALL BE RETAINED
- FOR STABILITY. (SEE STANDARD DETAIL)

 8 WHERE SIDES OF A CHB WALL ADJOIN A COLUMN OR RC WALL

 SEE CONTROL JOINT DETAIL AT STANDARD DETAILS. REBARS

 AS REQUIRED IN 10.6 SHALL BE RETAINED FOR STABILITY.

 FOR HIGH WALLS, LINTEL BEAMS INTERVAL SHALL BE PROVIDED AT 3.0 m.
- .9 NO CHIPPING OFF OF CONCRETE COLUMNS AND BEAMS ARE ALLOWED
 UNLESS OTHERWISE PERMITTED BY THE ENGINEER
 WHERE COLUMNS AND BEAMS HAVE BEEN POURED WITHOUT DOWELS FOR THE
 CHB WALL PROVIDE 12¢ DRILLED DOWELS @ 600mm O.C. WITH EPOXY GROUT

STRUCTURAL TOLERANCES_

UNLESS OTHERWISE SPECIFIED BY THE ENGINEER, THE FOLLOWING ARE THE ACCEPTABLE STRUCTURAL TOLERANCES FOR CAST—IN—PLACE CONCRETE CONSTRUCTION.

- CROSS SECTIONAL DIMENSIONS AND LOCATION TO REINFORCEMENT.

 DIMENSION LESS THAN = ± 6 mm
- 200 mm TO 600 mm = \pm 9 mr \pm 3. MEMBER LENGTH OR HEIGHT = \pm 12 mr
- (MAXIMUM LIMITATION = 12 mm)

 C. DEVIATION FROM STRAIGHT LINE = 6 mm PER 3.0 METERS
- (SWEEP AND OR PLUMBNESS)

 D. LOCATION OF BAR CUT-OFFS = ± 50 mm
 OR BENDS

CONCRETE PROTECTION FOR REINFORCEMENT_

THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING BARS. FOR BAR BUNDLES THE MINIMUM COVER SHALL EQUAL THE EQUIVALENT DIAMETER OF THE BUNDLE. BUT NEED NOT BE MORE THAN 50 mm ON THE TABULATED MINIMUM WHICHEVER IS GREATER.

1 CAST-IN-PLACE CONCRETE (REINFORCED CONCRETE CONSTRUCTION.)

A. CAST AGAINST AND PERMANENTLY MINIMUM COVER EXPOSED TO EARTH 75 mm

50 mm

20 mm

- B. EXPOSED TO EARTH OR WEATHER 20¢ AND LARGER
- 16ø AND SMALLER
 C. NOT EXPOSED TO WEATHER OR IN

NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND SLABS, WALL AND JOISTS

16¢ AND SMALLER

BEAMS,GIRDERS AND COLUMNS PRINCIPAL REINFORCEMENT, TIES

STIRRUPS OR SPIRALS
40 mm

REINFORCING BARS_

- 1 ALL MAIN REINFORCING BARS SHALL BE DEFORMED CONFORMING TO ASTM A615-68
- 2 UNLESS OTHERWISE NOTED IN THE PLANS THE MINIMUM YIELD STRENGTH OF THE MAIN REINFORCING BARS SHALL BE AS FOLLOWS:

12ø mm AND SMALLER fy = 40,000 psi 16ø mm AND LARGER fy = 60,000 psi

3 IN STANDARD DETAILS.

STANDARD HOOK

- 1 A STANDARD HOOK FOR REBARS IF REQUIRED SHALL BE EITHER OF THE FOLLOWING:
- A. A SEMI—CIRCULAR TURN PLUS AN EXTENSION OF AT LEAST 4 DIAMETER BUT NOT LESS THAN 62 mm AT THE FREE END OF THE BAR.
- B. A 90° TURN PLUS EXTENSION OF AT LEAST 12 BAR DIAMETERS AT THE FREE END OF THE BAR.
 - = 6 BAR DIAMETERS = 8 BAR DIAMETERS
- 2 MINIMUM DIAMETERS OF BEND MEASURED ON THE INSIDE OF THE BAR TO THE FREE END OF THE BAR.

10 mm TO 25 mm 28 mm TO 36 mm

CONSTRUCTION JOINTS_

- 1 CONSTRUCTION JOINTS SHALL BE LOCATED NEAR THE MIDDLE OF SPAN OF SLABS, BEAMS OR GIRDER
- 2 AT BEAM-GIRDER INTERSECTION, THE CONSTRUCTION JOINT ON THE GIRDER SHALL BE OFFSET A DISTANCE EQUAL TO TWICE THE WIDTH OF THE BEAM. DIAGONAL BARS SHALL BE PROVIDED TO RESIST 100 % SHEAR AT THE CONSTRUCTION JOINT.
 - THE TOTAL LENGTH OF THE DIAGONAL BAR SHALL BE AT LEAST TWICE THE DEVELOPMENT LENGTH REQUIRED FOR THE BAR. THE ENDS OF THE BAR SHALL BE HOOKED.
- 3 WHERE A JOINT IS TO BE MADE THE SURFACE OF THE CONCRETE SHALL BE THOROUGHLY CLEANED AND ALL LAITANCE AND STANDING WATER REMOVED. VERTICAL COATED WITH NEAT CEMENT GROUT IMMMEDIATELY BEFORE PLACING OF NEW CONCRETE.

ELECTRICAL CONDUITS_

- 1 ELECTRICAL CONDUITS MAY BE PLACED BETWEEN THE TOP AND BOTTOM REINFORCEMENT IN SLABS AND R.C. WALLS.
- 2 THE SPACING OF THESE CONDUITS SHALL BE NOT CLOSER THAN 3 BARS DIAMETER

PIPES EMBEDDED IN CONCRETE_

- 1 PIPES TO BE EMBEDDED ON COLUMNS SHALL BE SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER.
- 2 NO VERTICAL PIPES ARE ALLOWED TO PUNCH THROUGH A BEAM OR GIRDER.
- 3 HORIZONTAL PIPES MAY PUNCH THROUGH A BEAM OR GIRDER SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER.

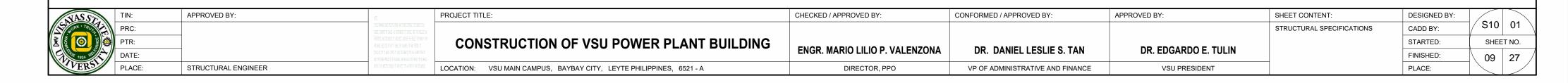
 THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER THE ACTUAL LOCATION WHERE PIPES CROSS A BEAM OR GIRDER, WHERE PERMITTED BY THE ENGINEER PROVIDE AT LEAST 16 mm (600 mm LONG) U—BARS TO ENCLOSE THE PIPE AT THE POINT OF INTERSECTION AT EACH FACE OF THE BEAM.
- 4 PIPES OF ALUMINUM SHALL NOT BE EMBEDDED IN CONCRETE.
- 5 CONCRETE COVER FOR PIPES SHALL BE AT LEAST 38 mm FOR CONCRETE SURFACE EXPOSED TO THE WEATHER OR IN CONTACT WITH THE GROUND 18 mm FOR CONCRETE SURFACE NOT EXPOSED TO THE GROUND OR WEATHER.

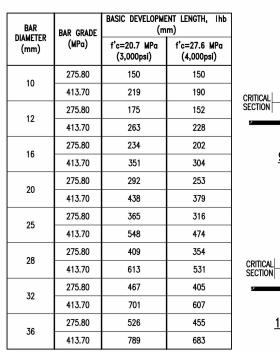
REMOVAL OF FORMS AND SHORES

- 1 FORMS SHALL BE REMOVED IN SUCH MANNER AS TO ENSURE COMPLETE SAFETY OF THE STRUCTURE.
- 2 UNLESS OTHERWISE SPECIFIED BY THE STRUCTURAL ENGINEER THE FOLLOWING SHALL BE THE BASIS OF FORMS AND SHORINGS REMOVAL FOR REINFORCED CONCRETE (R.C.) CONSTRUCTION.

BEAM SIDE FORMS
COLUMN SIDE FORMS
SLAB BOTTOM FORMS
BEAM SHORING
SLAB SHORING
BEAM SHORING
BEAM SIDE FORMS
DAYS
AGE OF CONCRETE
BEAM SHORING
BEAM SHORING
BEAM SIDE FORMS
DAYS
AGE OF CONCRETE

STRUCTURAL SPECIFICATIONS



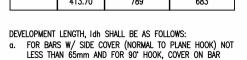


150 (MIN)

90° HOOK

150 (MIN)

180° HOOK



b. FOR BARS W/ HOOK ENCLOSED VERTICALLY OR OR HORIZONTALLY W/IN TIES OR STIRRUP TIES SPACED ALONG THE FULL DEVELOPMENT LENGTH Idh NOT GREATER THAN

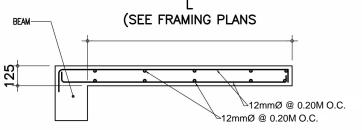
EXTENSION BEYOND HOOK NOT LESS THAN 50mm, USE 0.7

3db, USE 0.8 lhb.
c. FOR BARS NOT INCLUDED IN (a) & (b), USE 1.0 lhb.

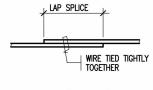
DEVELOPMENT LENGTH OF HOOKED BARS

		MINIMUM D	EVELOPME	ENT LENGTH	1 ld (mm)	
BAR DAIMETER (mm)	BAR GRADE (MPa)	f'c=20 (3,00		f'c=27 (4,00		
		TOP BARS	BARS	TOP BARS	BARS	
10	275.80	300	300	300	300	
10	413.70	427	329	370	300	
40	275.80	342	300	300	300	1d
12	413.70	513	394	444	342	300 (MIN
40	275.80	456	351	395	304	CRITICAL
16	413.70	684	526	592	455	SECTION db
00	275.80	570	438	493	379	<i>'</i>
20	413.70	854	657	740	569	<i> </i>
0.5	275.80	734	565	636	489	
25	413.70	1101	847	953	733	
	275.80	921	708	797	613	
28	413.70	1381	1062	1196	920	
70	275.80	1202	925	1041	801	
32	413.70	1804	1387	1562	1202	
70	275.80	1522	1171	1318	1014	
36	413.70	2283	1756	1977	1521	

DEVELOPMENT LENGTH OF STRAIGHT BARS



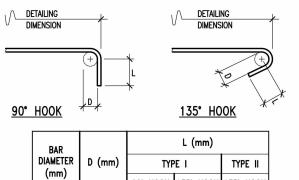
TYP. CANTIVELER SLAB DETAIL



OFFSET



OFFSET

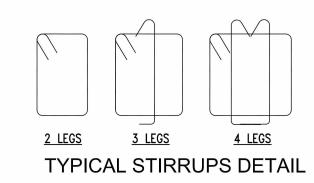


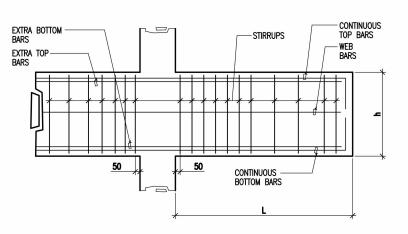
90° HOOK 135° HOOK 135° HOOK 12 NOTE: TYPE I FOR GENERAL USE
TYPE II FOR SEISMIC USE (FOR ALL COLUMNS AND ALL

BEAMS CONNECTED TO COLUMNS) **STIRRUPS** AND TIE HOOK DIMENSIONS

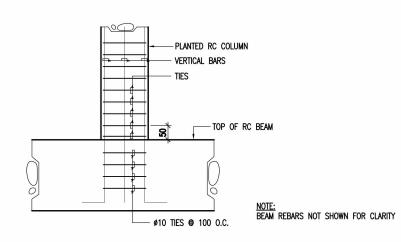
DIMENSION	nm)	L (n	D ()	BAR IAMETER	
(+)	90° HOOK	180° HOOK	D (mm)	(mm)	
	65 120 65 145 65 192		60	10	
D			72	12	
90° HOOK			96	16	
DETAILING	240	80	120	20	
DIMENSION	305	100	150	25	
D) (+	335	115	225	28	
\	385	130	255	32	
	144 432		288	36	

STANDARD HOOK DIMENSIONS

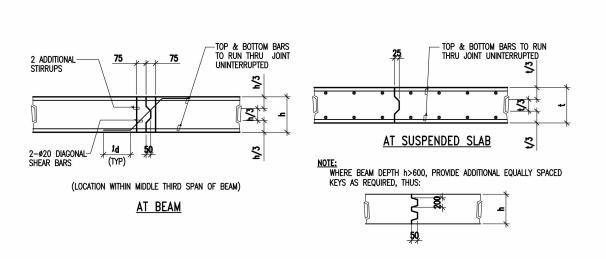




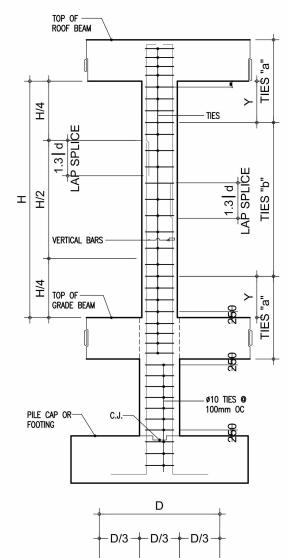
TYPICAL CANTILEVER BEAM DETAIL

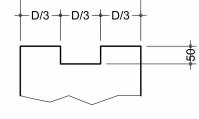


TYPICAL PLANTED RC COLUMN DETAIL



TYPICAL CONSTRUCTION JOINT DETAILS



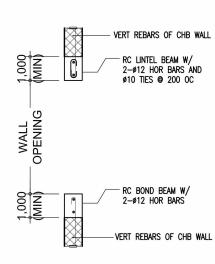


C.J. = CONSTRUCTION JOINT

NOTES: 1. Y=MAX OF THE FF:

- H/6 450 MM MAXIMUM COLUMN DIMENSION SPLICES ARE PERMITTED ONLY WITHIN THE CENTER HALF OF COLUMN HEIGHT (H) STAGGER BAR SPLICES BY 600 MM OR MORE
- PROMOTE TIES © 100 MM O.C. (MAX) OVER THE FULL LAP SPLICE LENGTH SPECIAL TIES © BEAM-COLUMN JOINT TO CONFORM TO THE SAME CONFIGURATION OF TIES AS INDICATED IN THE SCHEDULE OF COLUMNS

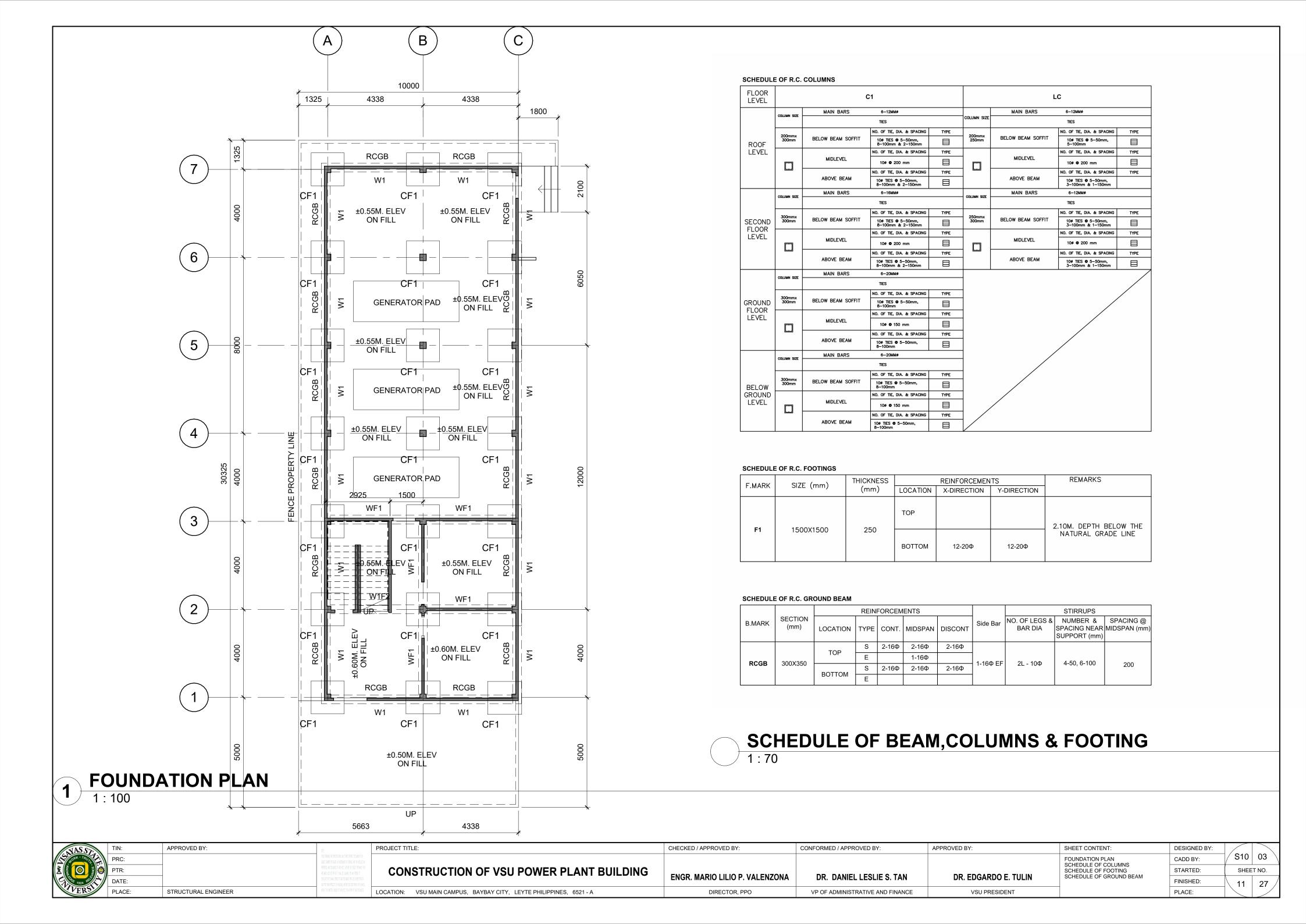
TYPICAL RC COLUMN ELEVATION

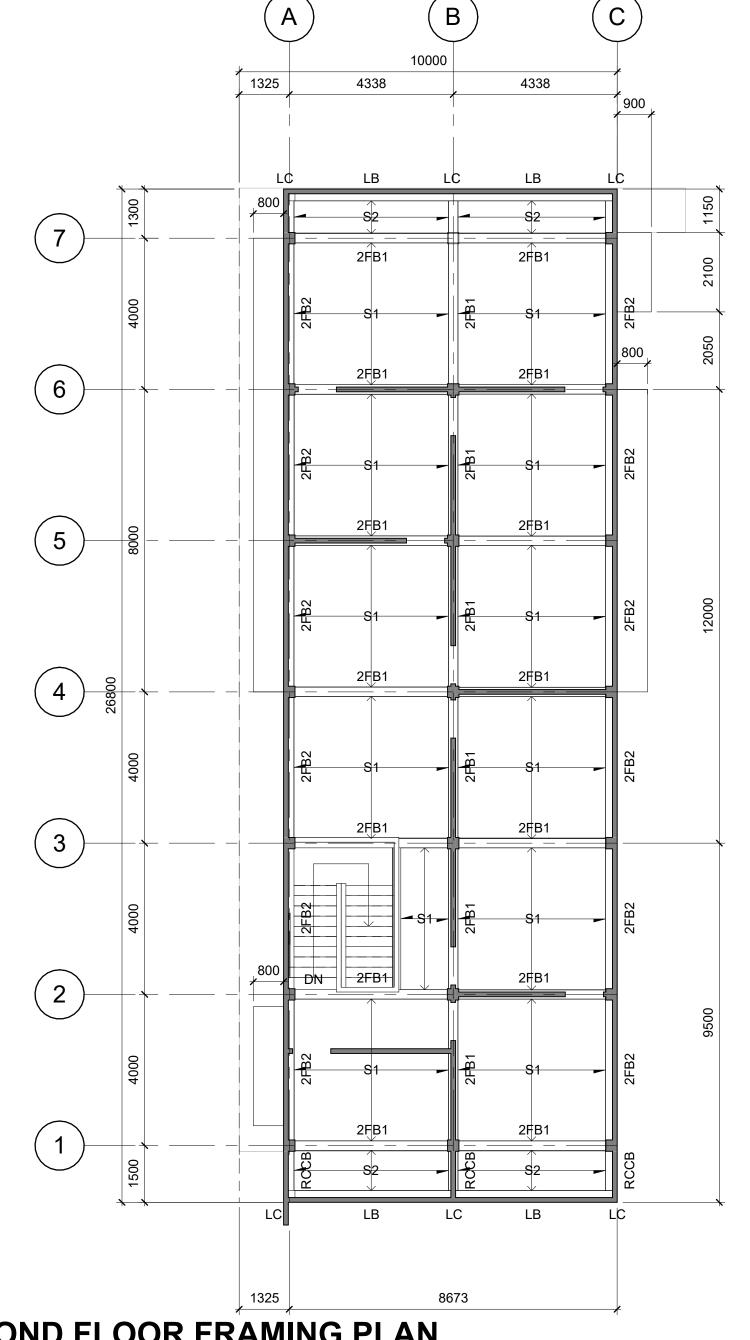


TYPICAL WALL OPENING DETAIL

STRUCTURAL DETAILS

NASST	TIN:	APPROVED BY:	VE	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	
S SORK TRUTH	PRC:		FEE DRING AN SECRETOR AND THE COURT DOUBLES DU Sobel, safet or sale, as instigles of socie, as he induction					STRUCTURAL DETAILS	CADD BY:	S10 02 \
King O	PTR:		PROPERTS AND DOCUMENTS OF ARCHIECT, MERIE THE CREAT FOR MICH THEY Are was decided in with 1 shall be warred and present to	CONSTRUCTION OF VSU POWER PLANT BUILDING	ENOR MARIO LILIO RIVALENZONA	DD DANIEL LEGILE O TAN			STARTED:	SHEET NO.
90F 192A RE	DATE:		DALCAT OF TO MAK COPES OF SAD DOCUMENTS FOR USE IN EXECUTION OF And for other process of bronders have december about on in mode		ENGR. MARIO LILIO P. VALENZONA	DR. DANIEL LESLIE S. TAN	DR. EDGARDO E. TULIN		FINISHED:	10 27
VERS	PLACE:	STRUCTURAL ENGINEER	MAND OF SUPPLY SUPERIOR OF SUPPLY SUP	LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A	DIRECTOR, PPO	VP OF ADMINISTRATIVE AND FINANCE	VSU PRESIDENT		PLACE:	





SCHEDULE OF SECOND FLOOR BEAMS

			RE	INFORCEM	1ENTS				STIRRUPS		
B.MARK	SECTION (mm)	LOCATION	TYPE	CONT.	MIDSPAN	DISCONT	Side Bar	NO. OF LEGS & BAR DIA	NUMBER & SPACING NEAR SUPPORT (mm)	SPACING @ MIDSPAN (mm)	
		TOP	S	2-20Ф	2-20Ф	2-20Ф					
2FB1	500X300	TOP	Е	2-20Ф		1-20Ф	1-16Ф EF	2L - 10Ф	4-50, 4-100	450	
ZFDI	3007300	воттом	S	2-20Ф	2-20Ф	2-20Ф	1-10Ψ ΕΓ	ΖΕ-10Φ	4-30, 4-100	150	
		BOTTOM	Е		2-20Ф						
		TOR	S	2-20Ф	2-20Ф	2-20Ф					
2FB2	400X300	TOP	Е				- 1-16Ф EF	2L - 10Ф	4-50, 4-100	450	
ZFB2	400/300	воттом	S	2-20Ф	2-20Ф	2-20Ф	1-10Ψ ΕΓ	ΖΕ - 10Φ	4-30, 4-100	150	
		BOTTOM	Е		1-20Ф						
		TOP	S	2-20Ф	2-20Ф	2-20Ф					
I D	2007200	TOP	Е				1 160 55	2L - 10Ф	4-50, 4-100	450	
LB	LB 300X200 -			S	2-16Ф	2-16Ф	2-16Ф	- 1-16Ф EF	ZL - 10Ψ	4-30, 4-100	150
			ВОТТОМ	Е		1-16Ф					

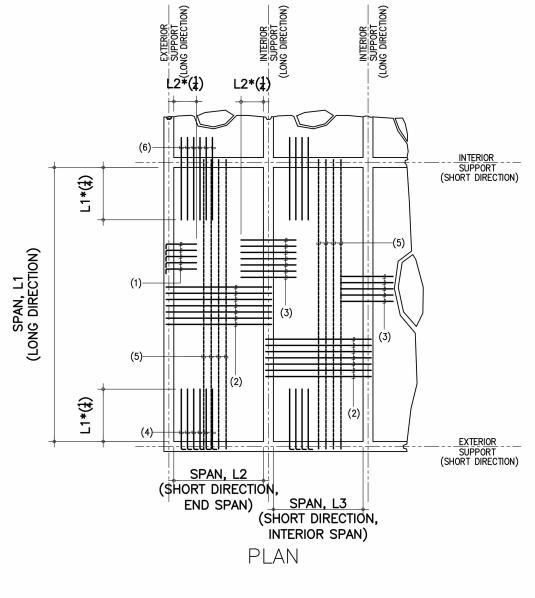
SCHEDULE OF SECOND FLOOR SLAB

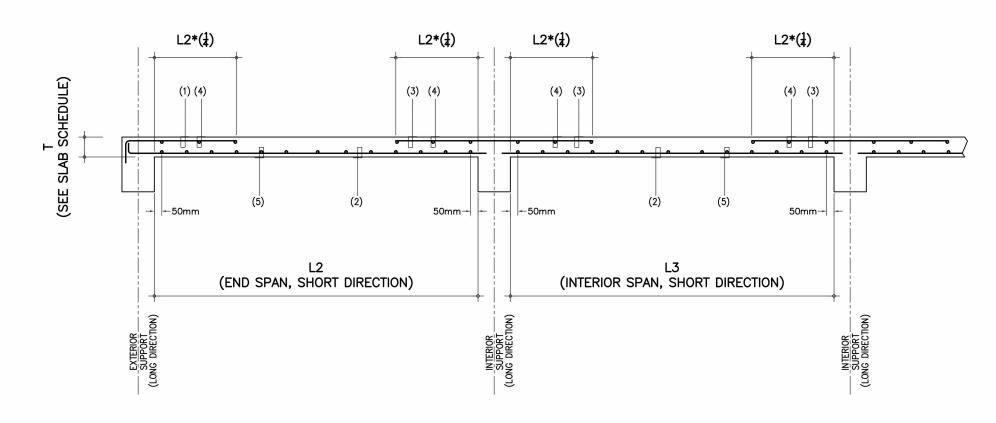
				REINFO	RCEMENTS					
S.MARK	THICKNESS (mm)		SHC	RT DIRECTION	ON	LON	G DIRECTIO	N	REMARKS	
	, ,	LOCATION	CONT	MIDSPAN	DISCON	CONT	MIDSPAN	DISCON		
S1	125	ТОР	12Ф @ 150			12Ф @ 200			TWO WAY	
31	125	воттом		12Ф @ 200	12Ф @ 200		12Ф @ 200	12Ф @ 200	100 000	
S2	125	ТОР	12Ф @ 125			PROVIDE 10	0Ф @ 300 DI	STRIBUTION	ONEWAY	
32	125	воттом		12Ф @ 150	12Ф @ 150	BARS			ONEWAT	
00	450	ТОР	12Ф @ 100			PROVIDE 1	0Ф @ 300 DI	STRIBUTION	CANTILEVER	
S3	150	воттом	10Ф @ 300			_ PROVIDE 10Φ @ 300 DISTRIBUTION BARS			CANTILEVER	

SCHEDULE OF FLOOR BEAMS & SLABS 1:160

SECOND FLOOR FRAMING PLAN

NASST	TIN:	APPROVED BY:	NE.	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	
S DORK TRUIT	PRC:		THE DRING AD SERVADIS AD THE COURAT DOUBLES DU! Sorb, same of sale, as isribus s frace, as the drilling					2ND FLOOR FRAMING PLAN	CADD BY:	S10 04 \
Koy O	PTR:		PROPRES AN OCCUPING OF ARCHES, DESERTE OF GREET FAT HICK THEY Are have secured in in it is shall be injured. Of an effolicit	CONSTRUCTION OF VSU POWER PLANT BUILDING	ENOD MADIO LILIO DI VALENZONA	DD DANIEL FOLIE 0 TAN	DD EDGADDO E TIII IN	SCHEDULE OF R.C. FLOOR BEAMS SCHEDULE OF R.C. SLABS	STARTED:	SHEET NO.
NASSTA BENEFIT OF THE PROPERTY OF THE PROPERT	DATE:		DIPLOPE OR TO WAR COPES OF SHO DICCIOLETS FOR ISE IN EPISTOD OF An art offer bounds, defer except a first of in hale		ENGR. MARIO LILIO P. VALENZONA	DR. DANIEL LESLIE S. TAN	DR. EDGARDO E. TULIN		FINISHED:	
VERSI	PLACE:	STRUCTURAL ENGINEER	HEALT HE HETEL CONSTITUTE ACHIECT OF A FIRST OF SAN DOCUMENTS.	LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A	DIRECTOR, PPO	VP OF ADMINISTRATIVE AND FINANCE	VSU PRESIDENT		PLACE:	

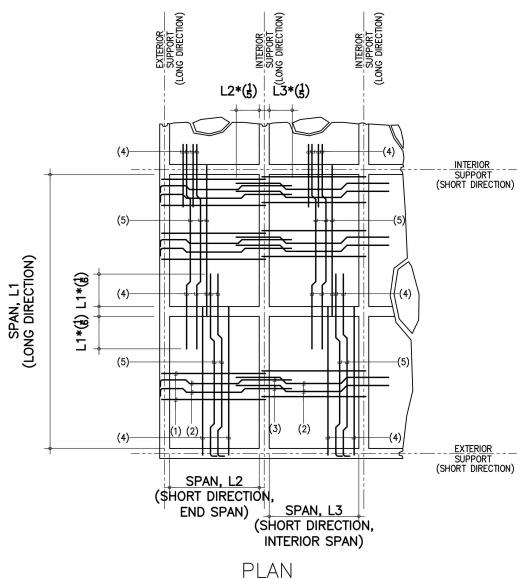


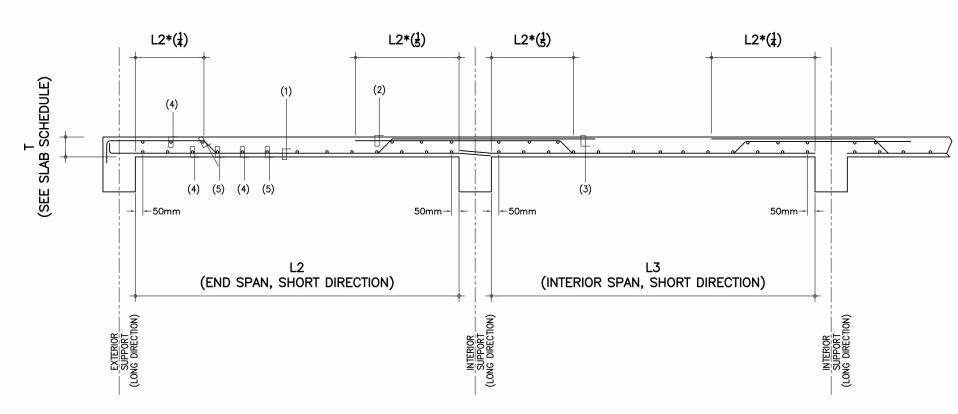


SECTION (ALONG SHORT DIRECTION)

TYPICAL ONE-WAY SLAB DETAIL

SCALE: NTS





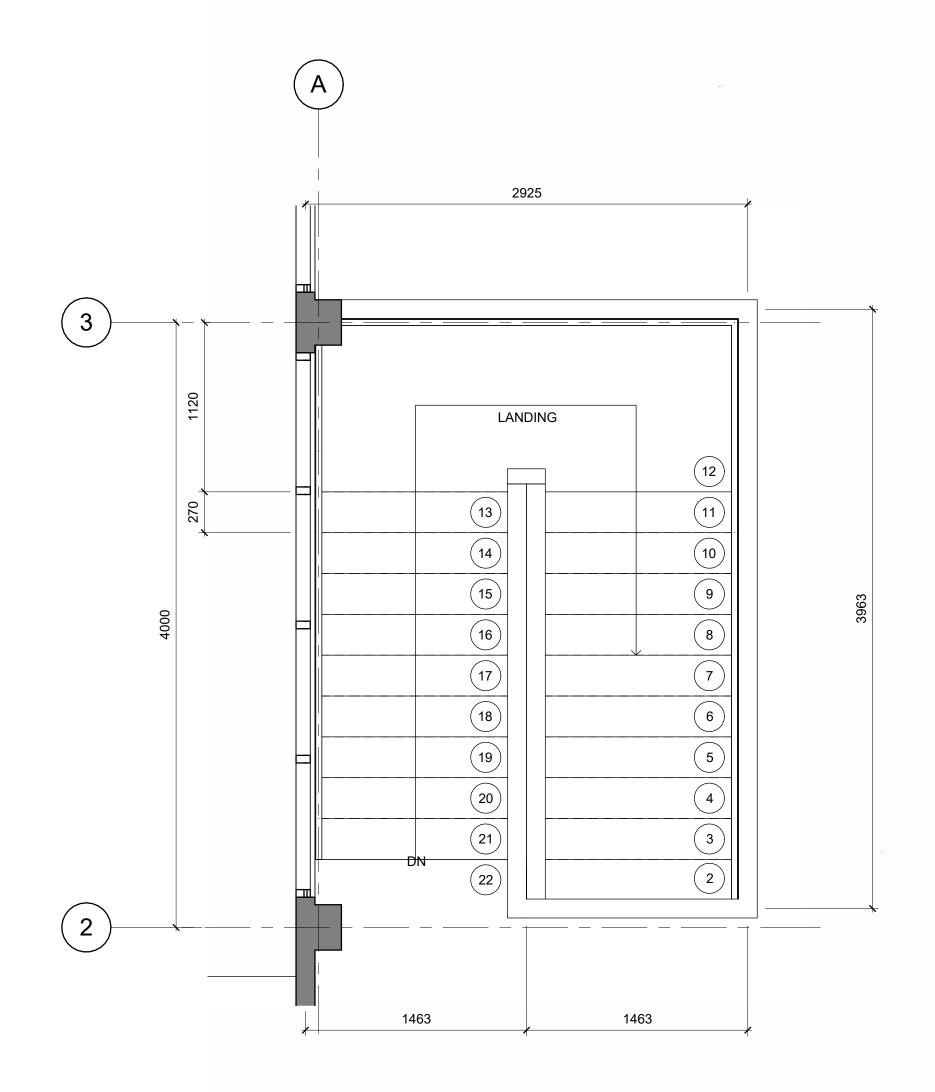
SECTION (ALONG SHORT DIRECTION)

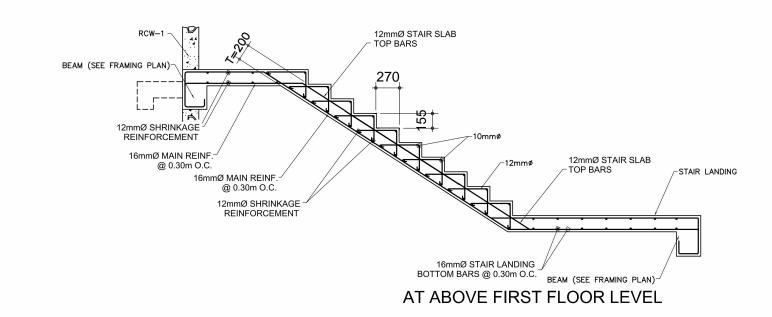
TYPICAL TWO-WAY SLAB DETAIL

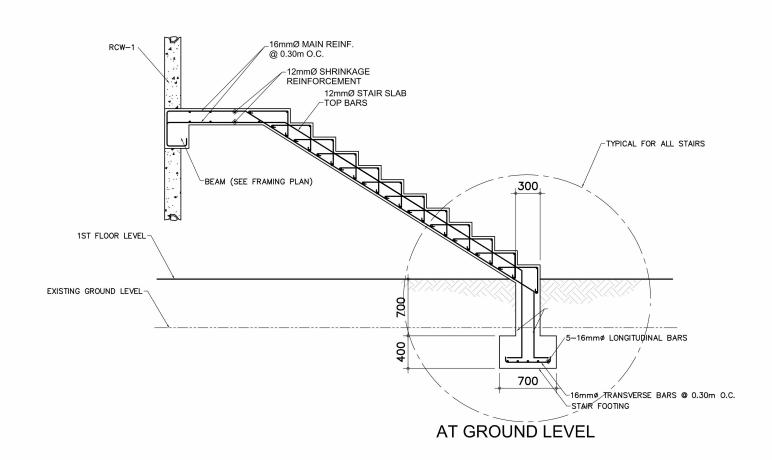
SCALE: NTS

DETAIL OF R.C. SLABS

CNASST	TIN:	APPROVED BY:	ITE	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	
S JORK · TRUTE	PRC:		HEE DRINGS AND SECRETARIS AND THEIR CONTRACT CONTRACT ON LINE School Samer of Search, as instructor of second and he induction					DETAIL OF R.C. FLOOR SLABS	CADD BY:	S10 05 \
Kana O O O O	PTR:		PRIMETES AND OCCURING OF ARCHECT, LEGER THE UNITAL THAT HAY AREA ARE LAKE EXCITED TO LITT IT SHALL BY LIMITED. IT AN TERSON TO	CONSTRUCTION OF VSU POWER PLANT BUILDING		DD DANIEL I EQUE 0 TAN			STARTED:	SHEET NO.
924 F	DATE:		ORDATE OR TO THE COPES OF SHO DOLINGS THE SECTION OF HIGH PROPERTY OF A LINEAR LINEAR SECTION FROM IN THE		ENGR. MARIO LILIO P. VALENZONA	DR. DANIEL LESLIE S. TAN	DR. EDGARDO E. TULIN		FINISHED:	13 27
VERSI	PLACE:	STRUCTURAL ENGINEER	UTROLT THE WATER COMEDITOR ACCHRECT OR ALPRAY OF SAN DOCUMENTS.	LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A	DIRECTOR, PPO	VP OF ADMINISTRATIVE AND FINANCE	VSU PRESIDENT		PLACE:	1



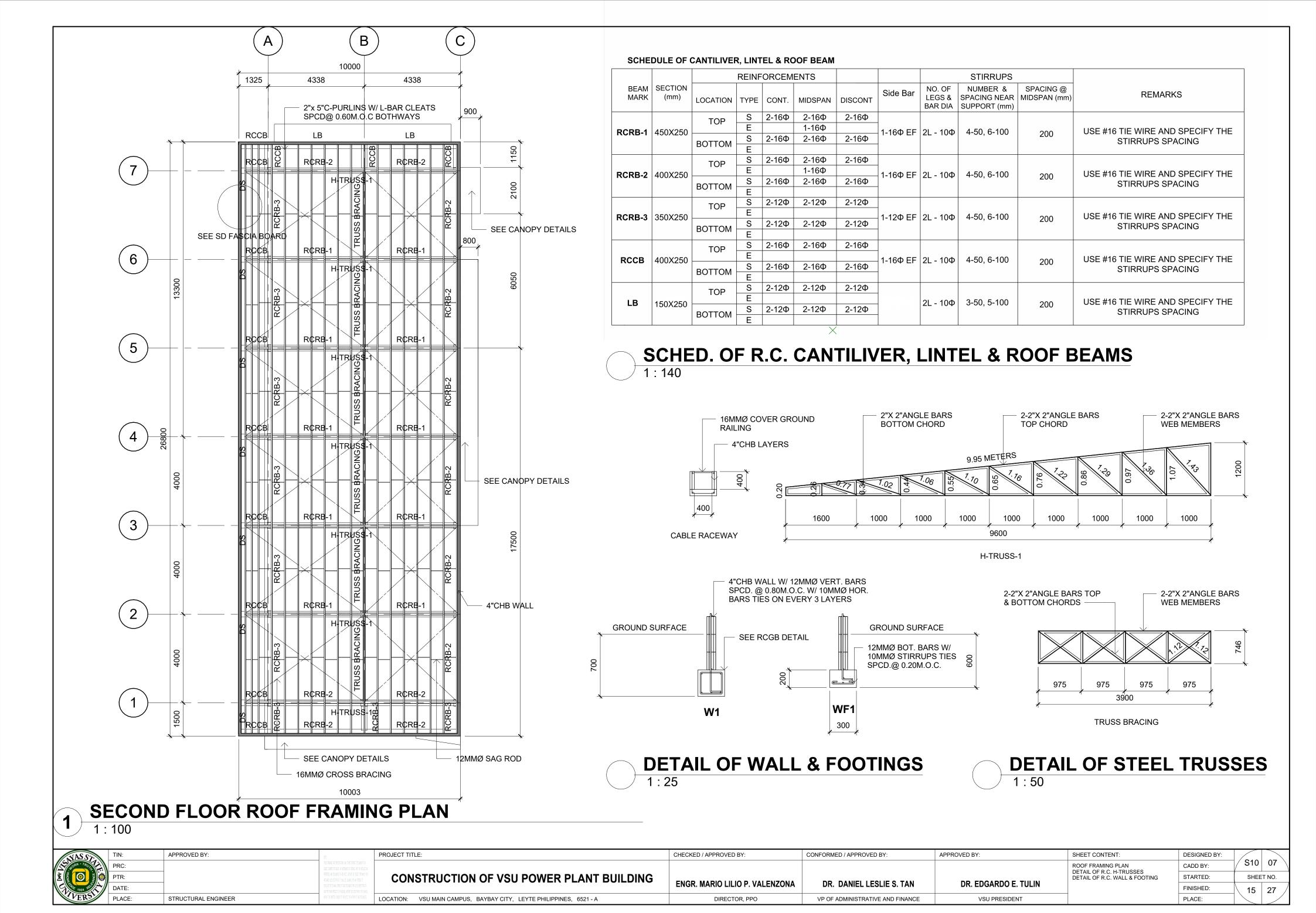


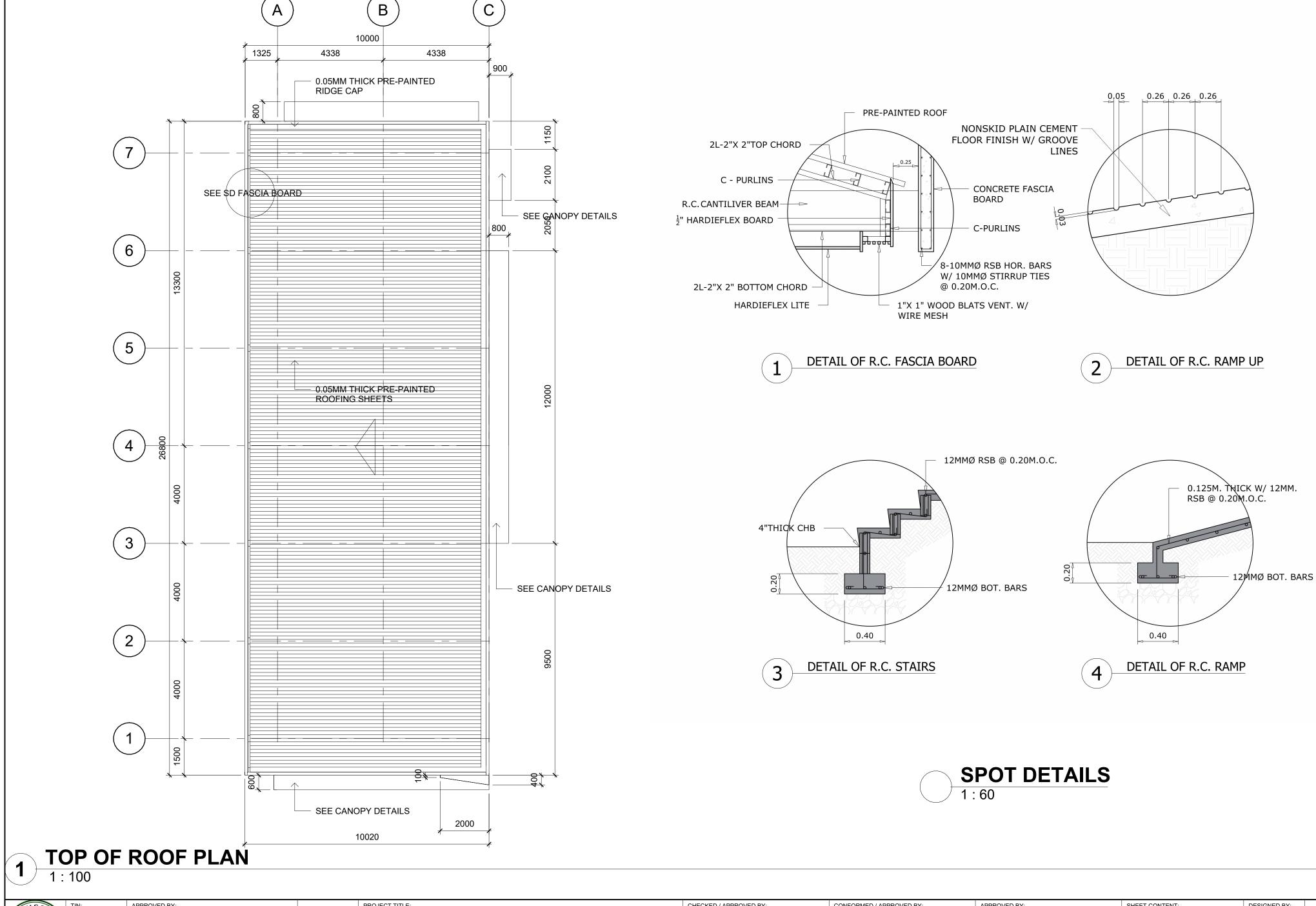


DETAIL OF R.C. STAIRS 1:25

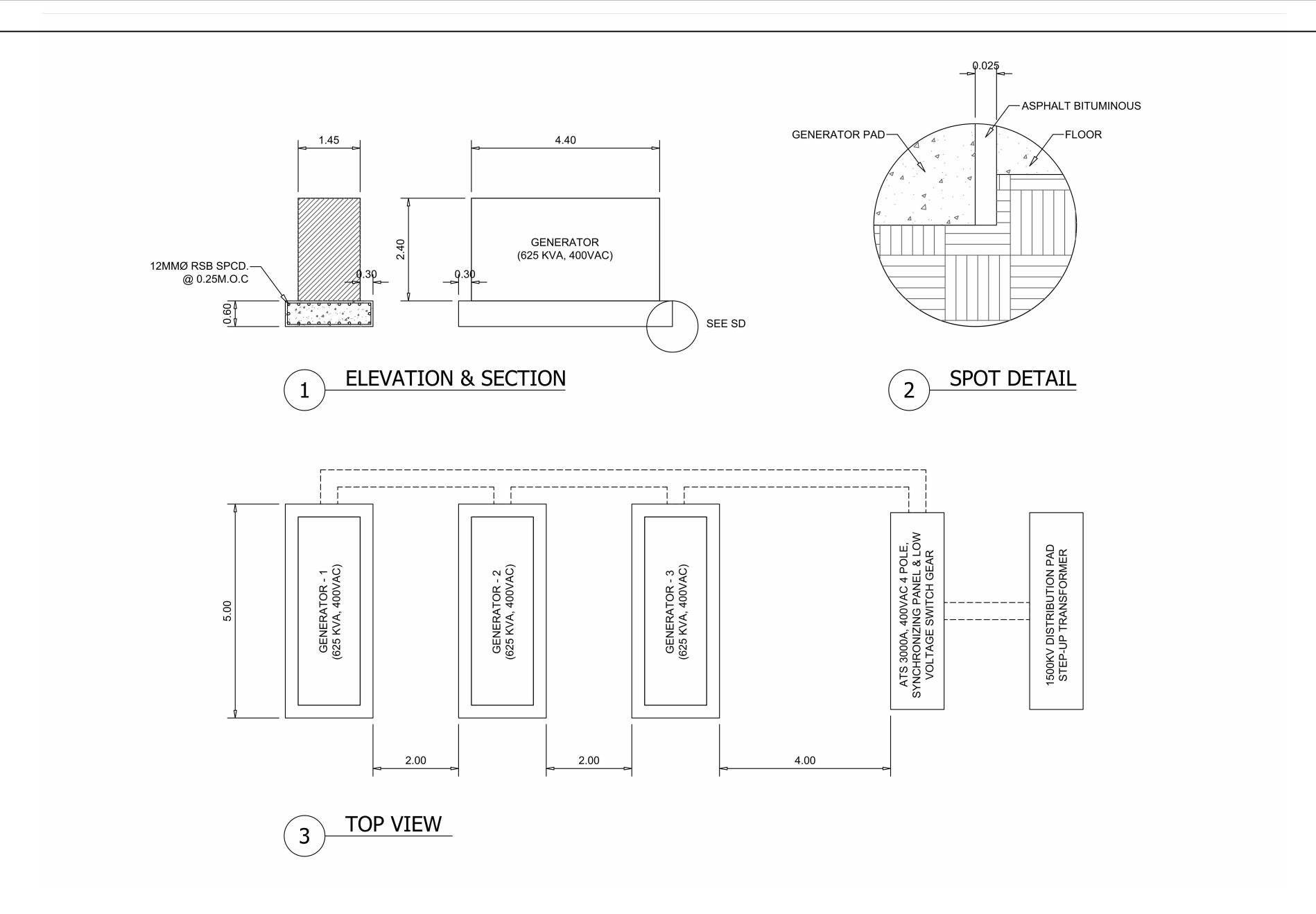
SECTION OF R.C. STAIRS

NASST	TIN:	APPROVED BY:	IVE	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	
S BORK · TRUIT	PRC:		THE DRAINS AND SECRETORS AND OTHER CONTRACT DOCUMENTS DILY Schel, Shaped or seale, as instrumous of serve, are the inelectival					DETAIL OF R.C. STAIRS SECTION OF R.C. STAIRS	CADD BY:	S10 06
T (No.)	PTR:		PROPERTY AND DOCUMENTS OF ARCHIEST, WEIGHT THE ORGET FOR HICH THEY are was executed or not it shall be unabard or any presson to	CONSTRUCTION OF VSU POWER PLANT BUILDING	ENOD MADIO LILIO DI VALENZONA	DD DANIEL LEGLIE O TAN	DD EDGADDO E TIII IN	SECTION OF R.C. STAIRS	STARTED:	SHEET NO.
G 1924 400	DATE:		OPECAE OR TO TAME OPES OF SAD DOCUMENTS FOR USE IN EXPENTION OF And for open projects of faunties, refer section parture in India.		ENGR. MARIO LILIO P. VALENZONA	DR. DANIEL LESLIE S. TAN	DR. EDGARDO E. TULIN		FINISHED:	14 27
VERS	PLACE:	STRUCTURAL ENGINEER	WHOLT HE WITEL CONSIST OF ACHTET OF ALFRIC OF SAD DOCKESTS.	LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A	DIRECTOR, PPO	VP OF ADMINISTRATIVE AND FINANCE	VSU PRESIDENT		PLACE:	



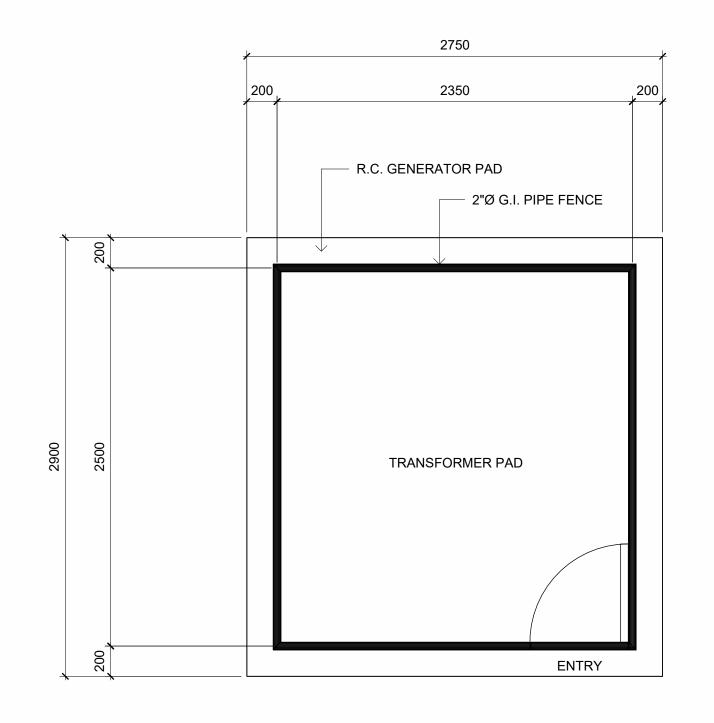


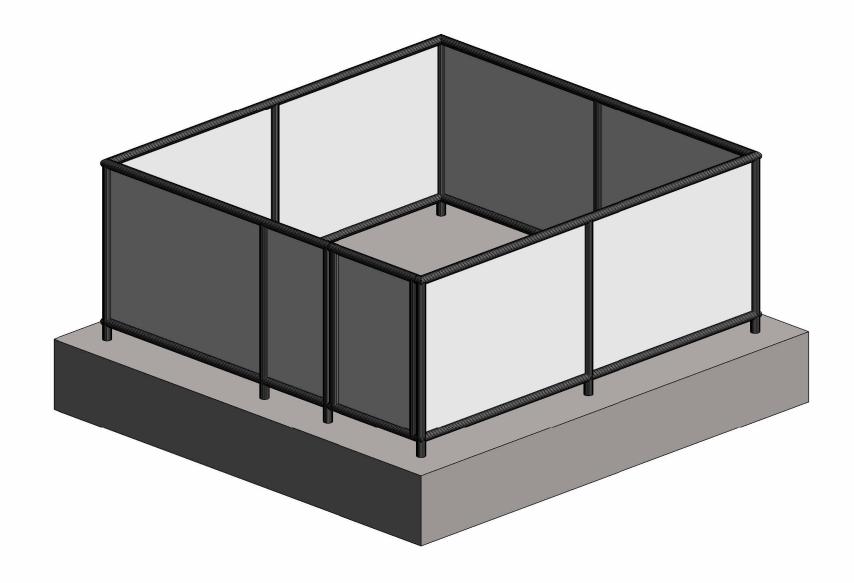
CHECKED / APPROVED BY: CONFORMED / APPROVED BY: APPROVED BY: PROJECT TITLE: APPROVED BY: SHEET CONTENT: DESIGNED BY: S10 08 TOP OF ROOF PLAN SPOT DETAILS CADD BY: CONSTRUCTION OF VSU POWER PLANT BUILDING STARTED: SHEET NO. ENGR. MARIO LILIO P. VALENZONA DR. DANIEL LESLIE S. TAN DR. EDGARDO E. TULIN FINISHED: 16 27 PLACE: STRUCTURAL ENGINEER LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A DIRECTOR, PPO VP OF ADMINISTRATIVE AND FINANCE VSU PRESIDENT PLACE:



DETAIL OF GENERATOR PAD

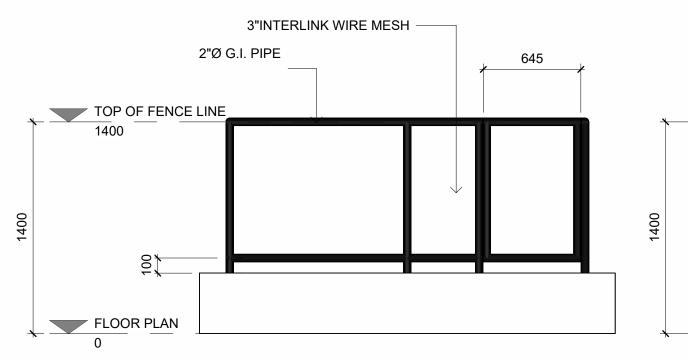
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S AORK TRUTE	PRC:		THEE DRAINS AN PREPIATION AND THEF CONTRACTION HEIS DUY sore, safet in sael, as instituts of sance, are in relectiva					DETAIL OF R.C. GENERATOR PAD	CADD BY:	S10 09 \
水 (No No N	PTR:		PROPRIES AN ORDINALS OF ACHIET, KEINE THE GLEET AR HICH THE ARE HAVE EXCURED ON OUT I SHALL BE HAVARLOR AN THROAD TO	CONSTRUCTION OF VSU POWER PLANT BUILDING	ENOD MADIO I II IO D VALENZONA	DD DANIEL LEGUE O TAN	DD 5004000 5 THE IN		STARTED:	SHEET NO.
G 1924 86 1	DATE:		TUPLATE ON TO WARE COPYES OF SHO DOCUMENTS FOR USE IN EXPERTION OF AND FOR OHIE PROJECTS OF THICKINGS, HEAVER EXECUTED PARTLY ON IN HALE		ENGR. MARIO LILIO P. VALENZONA	DR. DANIEL LESLIE S. TAN	DR. EDGARDO E. TULIN		FINISHED:	17 27
VERSI	PLACE:	STRUCTURAL ENGINEER	HEALT HE HETEL CONSET OF ACHTECT OF ALFREY OF SAD DOCUMENTS.	LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A	DIRECTOR, PPO	VP OF ADMINISTRATIVE AND FINANCE	VSU PRESIDENT		PLACE:	



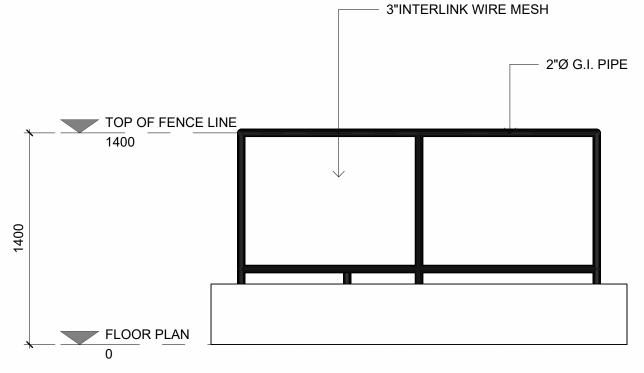


1 FLOOR PLAN 1: 25

5 3D_TRANSFORMER PAD



FRONT ELEVATION
1:25



4 REAR ELEVATION
1:25

TOP OF FENCE LINE

1400

FLOOR PLAN

0

3"INTERLINK WIRE MESH

2 RIGHT & LEFT SIDE ELEVATION 1:25

NAS ST	TIN:	APPROVED BY:	NE:	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	
S JORK · TRUTE	PRC:		PEE DRINS AD SECRETOR AD THE COURAT DOUBLES DUY Sole, stafe of sale, as infinists & spoc, ar he nelectia.					DETAIL OF TRANSFORMER PAD	CADD BY:	S10 10
太 (Nov) (O) (Nov) (O)	PTR:		PROPERES AND OCCUPIES OF ACCHEST, NETHER THE GREET FOR WHICH THEY are ware elected or not o shall be unlabel or any person to	CONSTRUCTION OF VSU POWER PLANT BUILDING	ENOD MADIO I II IO D MAI ENTONA	DD DANIEL LEGILE O TAN			STARTED:	SHEET NO.
G 1924 E 1924	DATE:		DIPLICATE OR TO WAS COPES OF SAID DOCUMENTS FOR MS: IN EXPENTION OF AND FOR OPER PROJECTS OF BINDINGS, HETHER SECURED FARTLY OR IN WHILE		ENGR. MARIO LILIO P. VALENZONA	DR. DANIEL LESLIE S. TAN	DR. EDGARDO E. TULIN		FINISHED:	18 27
VVERSI	PLACE:	STRUCTURAL ENGINEER	WHOIT THE WRITEN CONSIST OF ARCHIECT OF AIRMA OF SAD DOCUMENTS.	LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A	DIRECTOR, PPO	VP OF ADMINISTRATIVE AND FINANCE	VSU PRESIDENT		PLACE:	