



PERSPECTIVE

1 : 500

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REPUBLIC OF THE PHILIPPINES
DEPARTMENT OF PUBLIC WORKS AND HIGHWAYS
OFFICE OF THE BUILDING OFFICIAL
CITY OF BAYBAY
BAYBAY CITY

DISTRICT / MUNICIPALITY / CITY

LAND USE AND ZONING

APPROVED DATE

LINE AND GRADE

APPROVED DATE

ARCHITECTURAL

APPROVED DATE

SANITARY

APPROVED DATE

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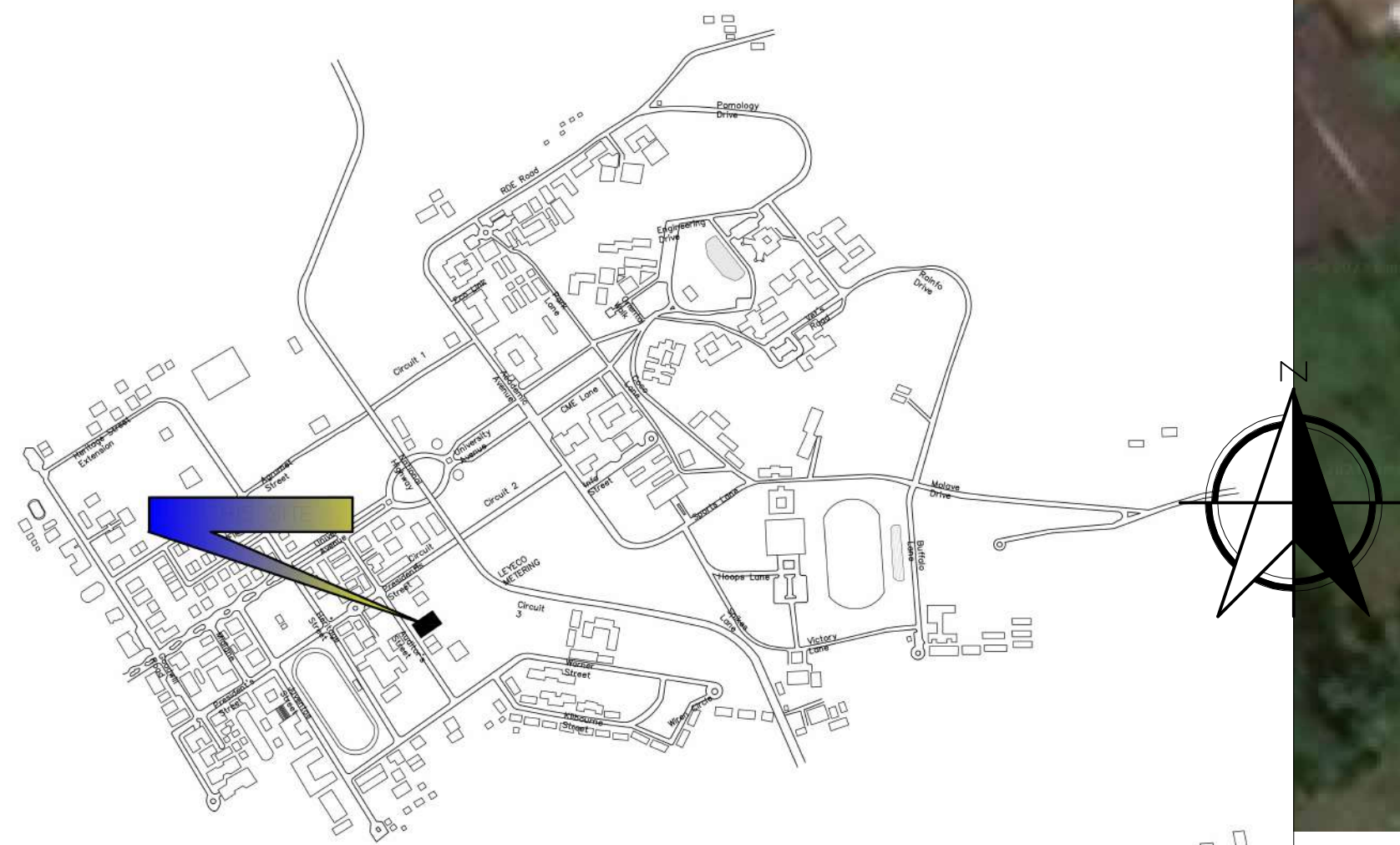
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MECHANICAL

APPROVED DATE

FIRE DEPARTMENT

APPROVED DATE



VICINITY MAP

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SITE DEVELOPMENT PLAN

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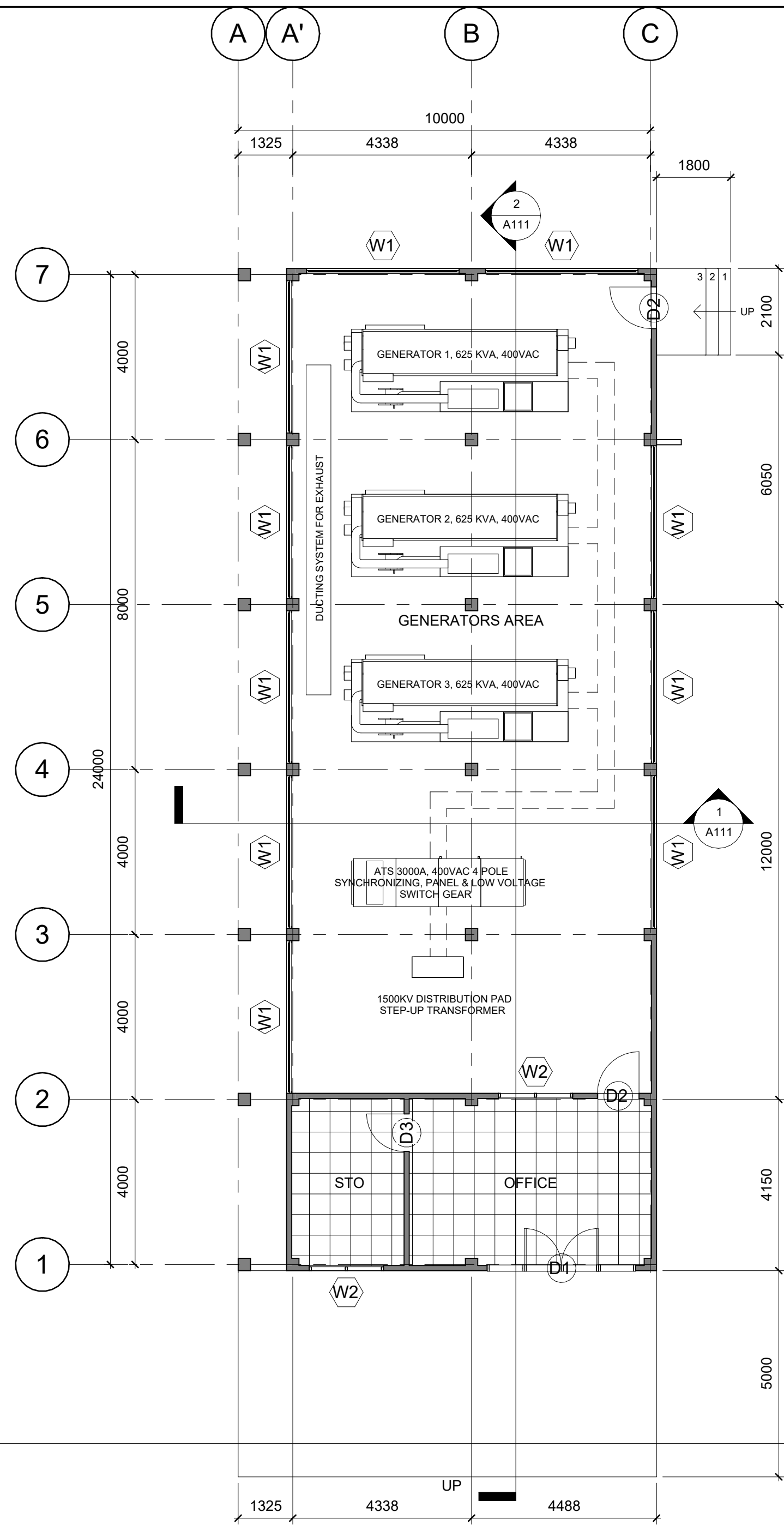
PROJECT TITLE:	CONSTRUCTION OF VSU POWER PLANT BUILDING
LOCATION:	VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A

CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:
ENGR. MARIO LILIO P. VALENZONA	DR. DANIEL LESLIE S. TAN
DIRECTOR, PPO	VP OF ADMINISTRATIVE AND FINANCE

APPROVED BY:
DR. EDGARDO E. TULIN
VSU PRESIDENT

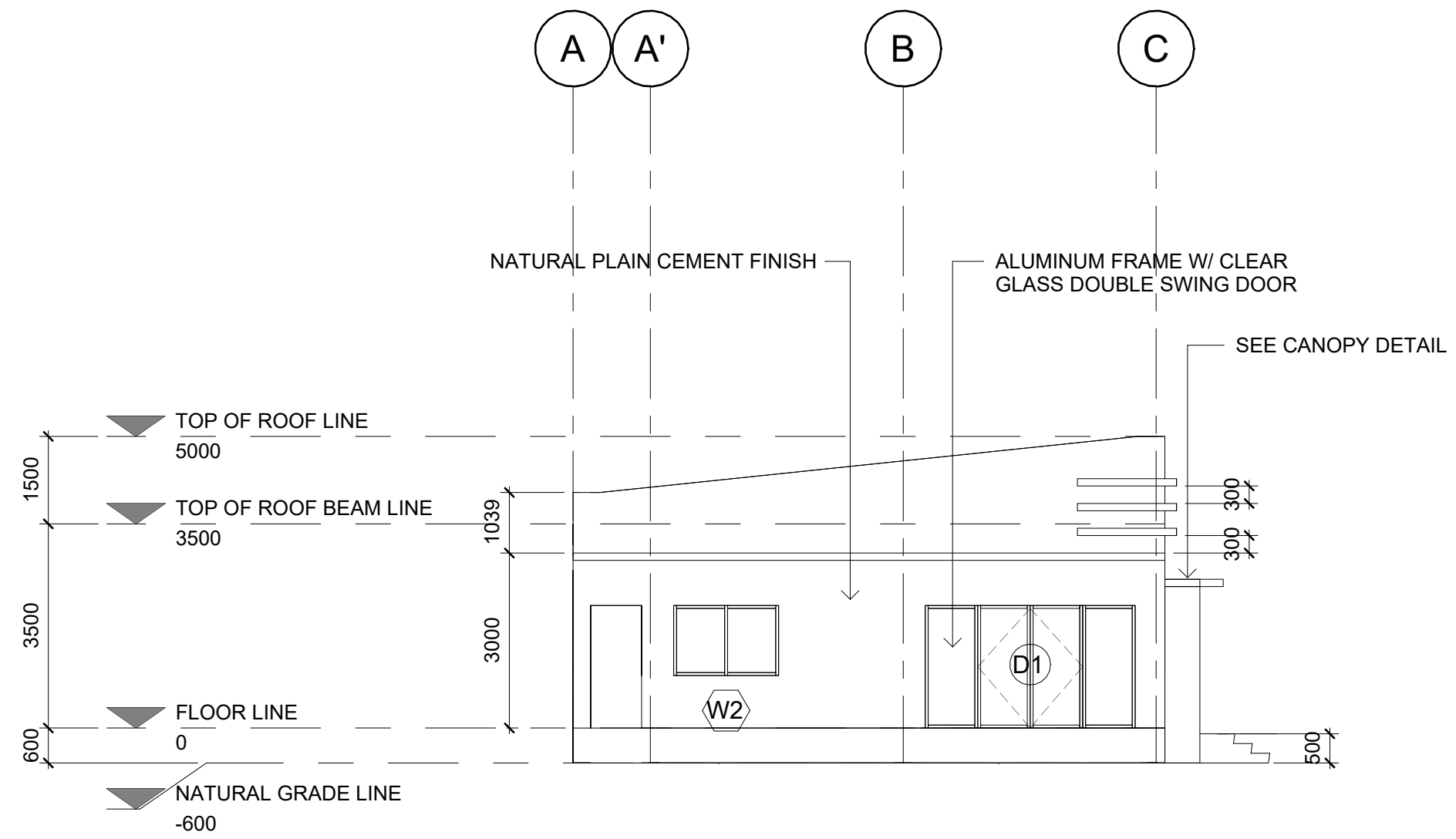
SHEET CONTENT:
PERSPECTIVE
VICINITY MAP
SITE DEVELOPMENT PLAN

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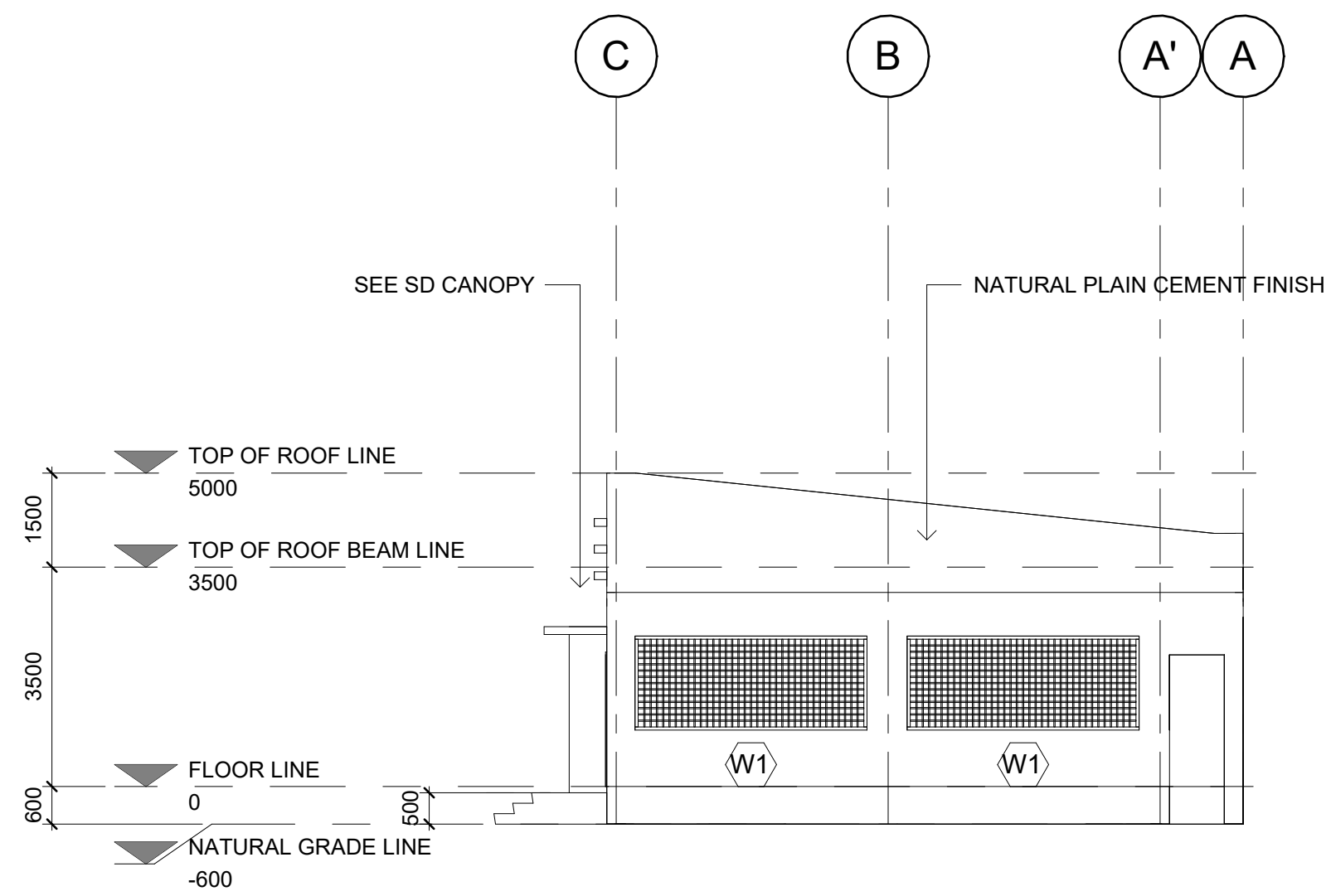


1 FLOOR PLAN
1 : 100


	TIN:	APPROVED BY:	<small> 1. SEE PLANS AND SPECIFICATIONS FOR DIMENSIONS AND NOTES. 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES. 3. ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE LATEST REVISIONS OF THE SPECIFICATIONS AND PLANS. 4. THE CONTRACTOR SHALL MAINTAIN ACCESS TO ALL ADJACENT PROPERTIES AT ALL TIMES. 5. ALL MATERIALS AND WORKMANSHIP SHALL BE SUBJECT TO INSPECTION AND APPROVAL BY THE ARCHITECT. </small>	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	<table border="1"> <tr> <td>A7</td> <td>02</td> </tr> <tr> <td colspan="2">SHEET NO.</td> </tr> <tr> <td>02</td> <td>16</td> </tr> <tr> <td colspan="2">PLACE:</td> </tr> </table>	A7	02	SHEET NO.		02	16	PLACE:	
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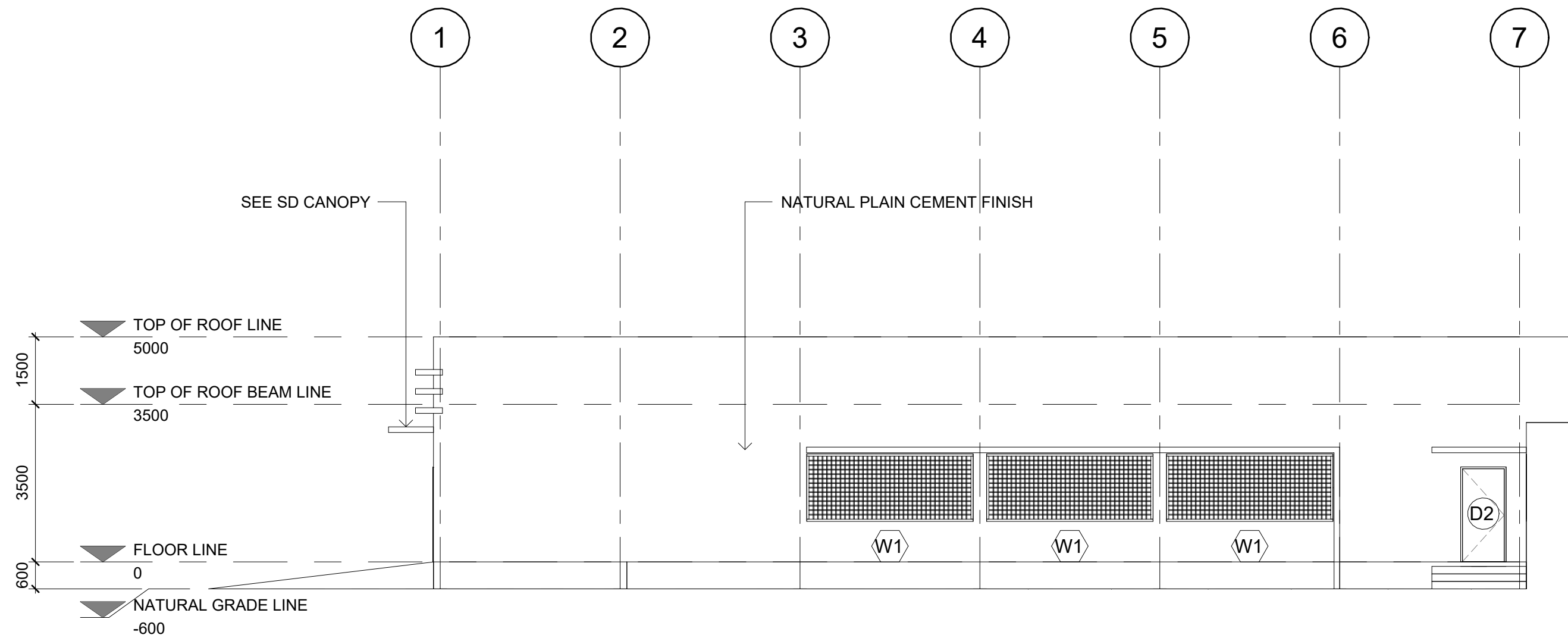


1 FRONT ELEVATION
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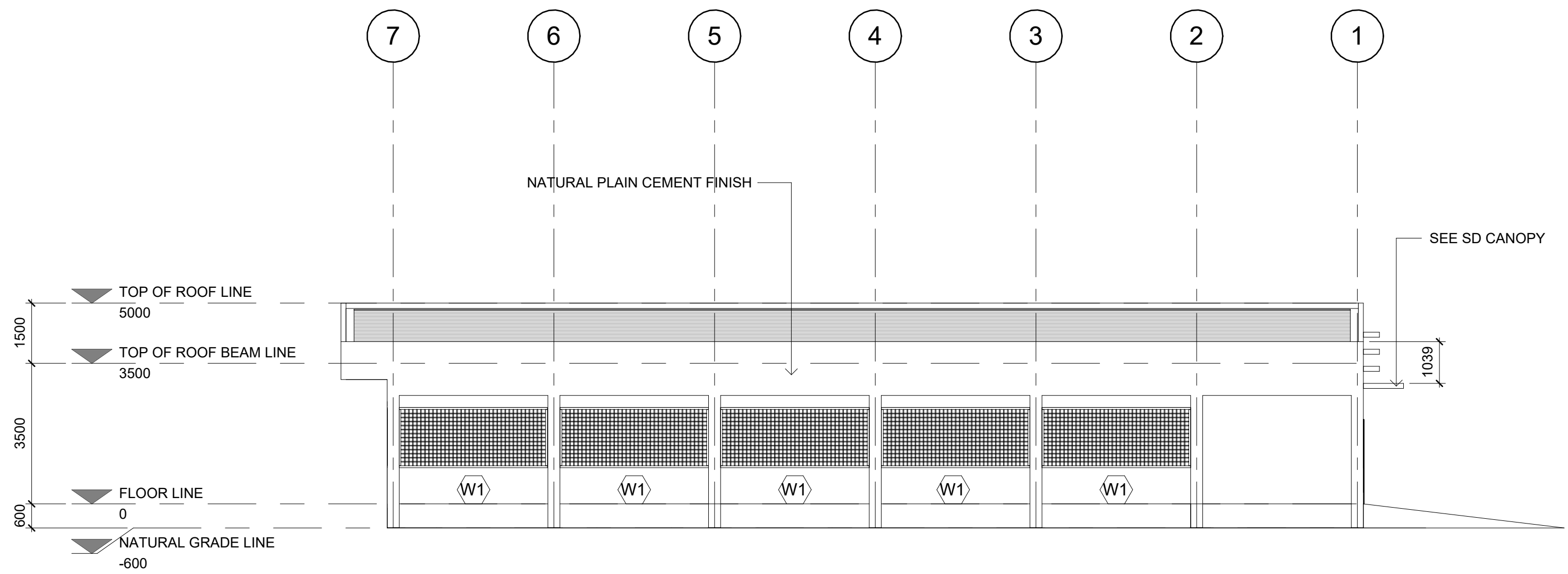


2 REAR ELEVATION
1 : 100

	TIN:	APPROVED BY:	<small> 1. SEE STANDARD SPECIFICATIONS AND DRAWINGS FOR MATERIALS AND METHODS OF CONSTRUCTION. 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES. 3. THE CONTRACTOR SHALL MAINTAIN ACCESS TO ALL ADJACENT PROPERTIES AT ALL TIMES. 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTING ALL UTILITIES AND EXISTING STRUCTURES. 5. THE CONTRACTOR SHALL MAINTAIN A NEAT AND SAFE WORKING SITE AT ALL TIMES. </small>	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	<table border="1"> <tr> <td>A7</td> <td>03</td> </tr> <tr> <td colspan="2">SHEET NO.</td> </tr> <tr> <td>03</td> <td>16</td> </tr> <tr> <td colspan="2">PLACE:</td> </tr> </table>	A7	03	SHEET NO.		03	16	PLACE:	
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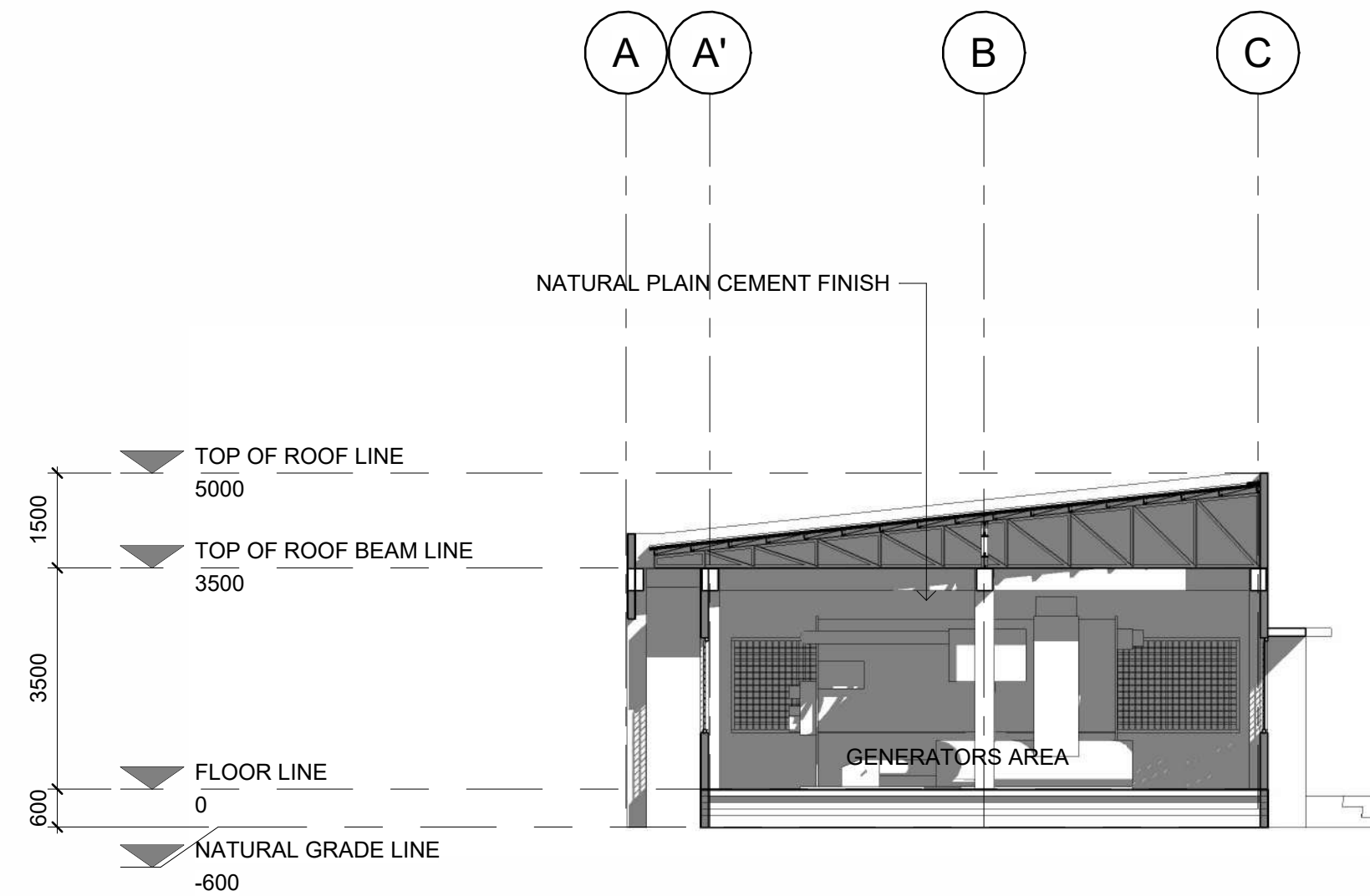


1 RIGHT SIDE ELEVATION
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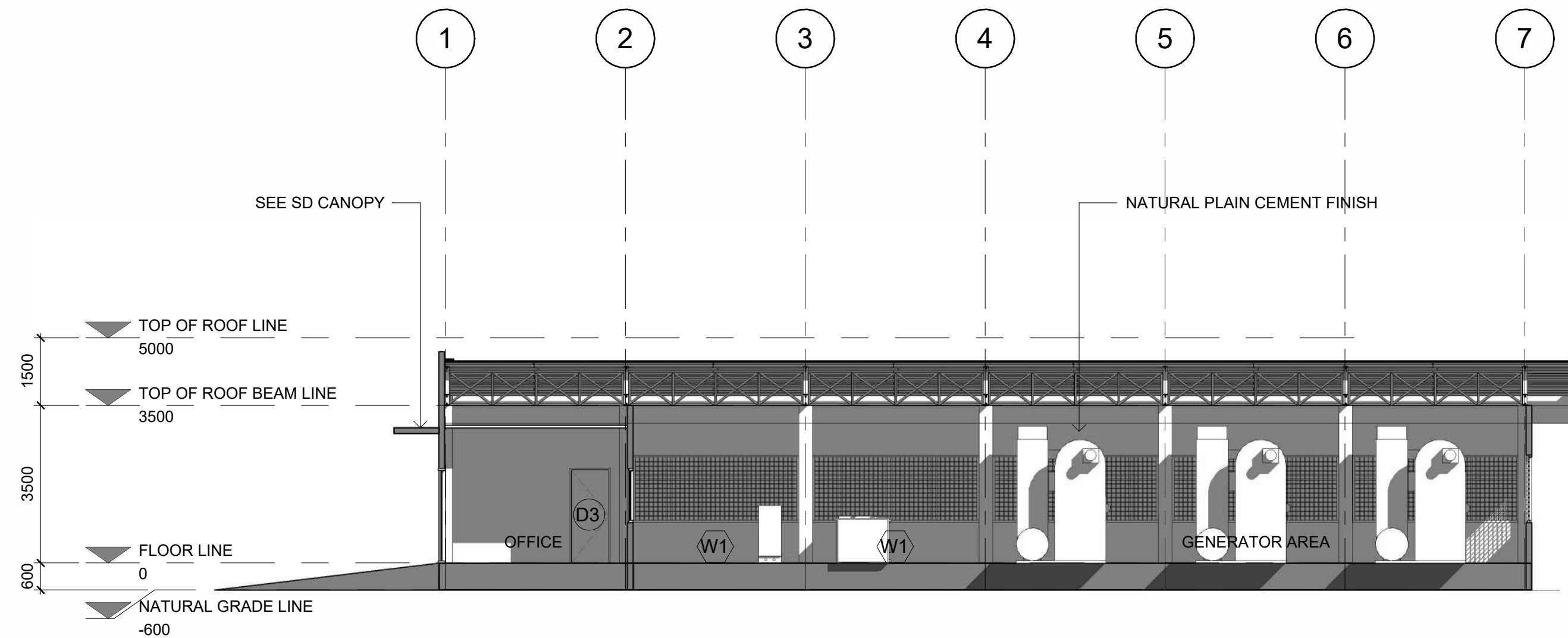


2 LEFT SIDE ELEVATION
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	TIN:	APPROVED BY:	<small> 1. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE SPECIFIED. 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE APPROPRIATE AGENCIES. 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING UTILITIES AND STRUCTURES. 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING TREES AND LANDSCAPE. 5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING ROADS AND HIGHWAYS. 6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING POWER LINES AND TELEPHONE LINES. 7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING WATER LINES AND SEWER LINES. 8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING CEMENT PAVEMENT AND ASPHALT PAVEMENT. 9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING CURBS AND GUTTERS. 10. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING SIGNAGE AND MARKINGS. 11. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING UTILITIES AND STRUCTURES. 12. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING TREES AND LANDSCAPE. 13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING ROADS AND HIGHWAYS. 14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING POWER LINES AND TELEPHONE LINES. 15. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING WATER LINES AND SEWER LINES. 16. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING CEMENT PAVEMENT AND ASPHALT PAVEMENT. 17. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING CURBS AND GUTTERS. 18. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROTECTION OF ALL EXISTING SIGNAGE AND MARKINGS. </small>	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	<table border="1"> <tr> <td>A7</td> <td>04</td> </tr> <tr> <td colspan="2">SHEET NO.</td> </tr> <tr> <td>04</td> <td>16</td> </tr> <tr> <td colspan="2">PLACE:</td> </tr> </table>	A7	04	SHEET NO.		04	16	PLACE:	
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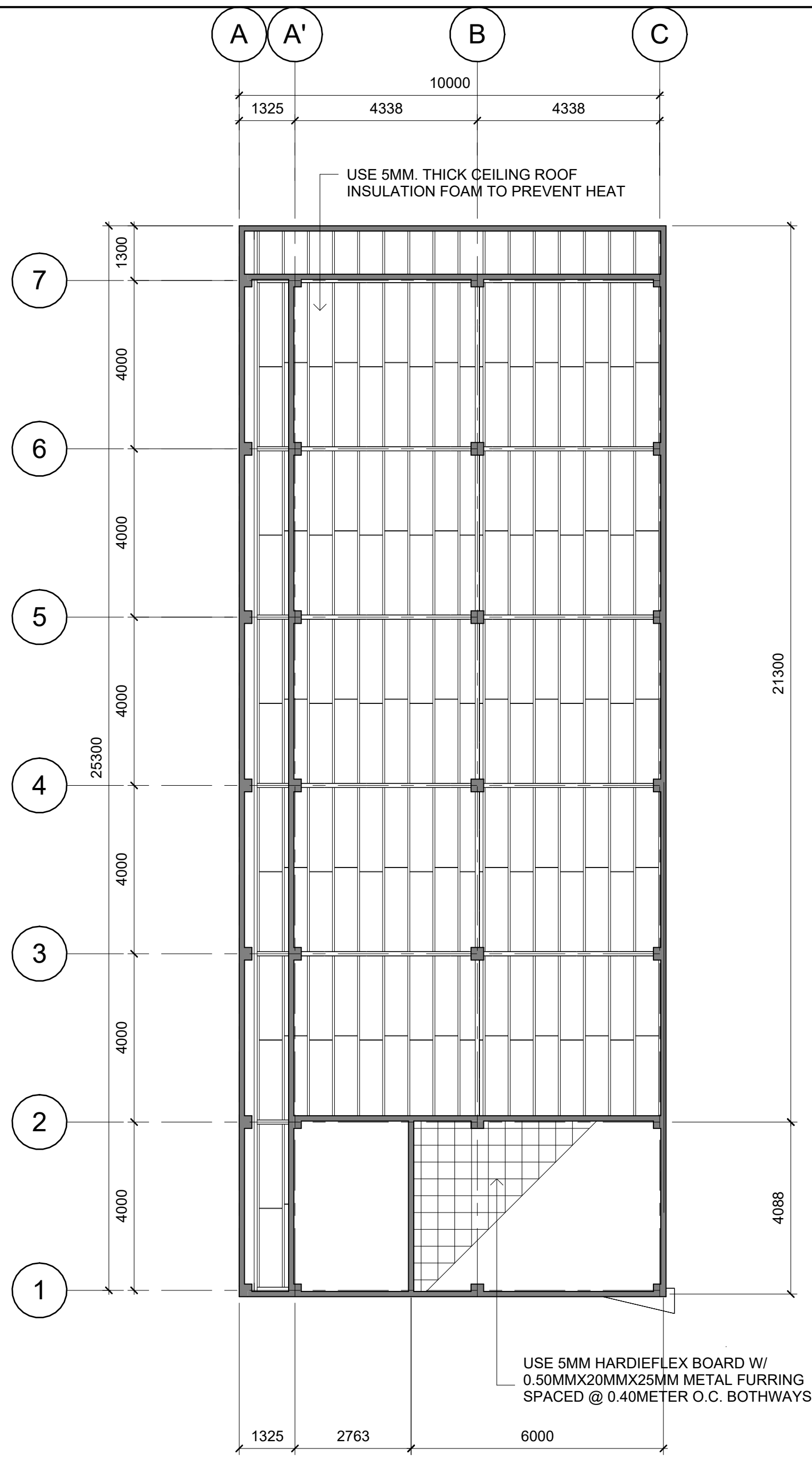


1 CROSS SECTIONAL - XX
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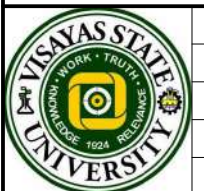


2 LONGITUDINAL SECTION - YY
1 : 100

	TIN:	APPROVED BY:	<small> 1. SEE GENERAL NOTES FOR PROJECT INFORMATION. 2. THIS DRAWING IS THE PROPERTY OF THE ARCHITECT. 3. NO PART OF THIS DRAWING IS TO BE REPRODUCED OR TRANSMITTED IN ANY FORM OR BY ANY MEANS, ELECTRONIC OR MECHANICAL, INCLUDING PHOTOCOPYING, RECORDING, OR BY ANY INFORMATION STORAGE AND RETRIEVAL SYSTEM, WITHOUT THE WRITTEN PERMISSION OF THE ARCHITECT. </small>	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	<table border="1"> <tr> <td>A7</td> <td>05</td> </tr> <tr> <td colspan="2">SHEET NO.</td> </tr> <tr> <td>05</td> <td>16</td> </tr> <tr> <td colspan="2">PLACE:</td> </tr> </table>	A7	05	SHEET NO.		05	16	PLACE:	
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DATE:						PLACE:												
PLACE:	ARCHITECT OF RECORD																	



1 REFLECTED CEILING PLAN
1 : 100

	TIN:	APPROVED BY:	<small>IS: THE ENGINEER OR ARCHITECT WHOSE NAME IS SET FORTH ON THESE PLANS IS RESPONSIBLE FOR THE ACCURACY OF THE INFORMATION CONTAINED HEREIN AND FOR THE DESIGN AND CONSTRUCTION OF THE PROJECT AS SHOWN ON THESE PLANS. ANY CHANGES TO THE PROJECT SHALL BE MADE BY A WRITTEN ORDER FROM THE ENGINEER OR ARCHITECT.</small>	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	<table border="1"> <tr> <td>A7</td> <td>06</td> </tr> <tr> <td colspan="2">SHEET NO.</td> </tr> <tr> <td>06</td> <td>16</td> </tr> <tr> <td colspan="2">PLACE:</td> </tr> </table>	A7	06	SHEET NO.		06	16	PLACE:	
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SCHEDULE OF DOORS			
NO.OF DOORS	D1	D2	D3
FRONT ELEVATION			
NO.OF SETS	1	2	1
REMARKS	FOUR PANEL W/ TWO - SWING GLASS DOOR W/ 2"X 5" ALUMINUM FRAME	SINGLE PANEL SWING FLUSH DOOR W/ 2"X 5" WOODEN DOOR JAMB	SINGLE PANEL SWING FLUSH DOOR W/ CLEAR GLASS SEE THRU W/ 2"X 5" WOODEN DOOR JAMB

DOOR LEGEND

1 : 50

SCHEDULE OF WINDOWS			
NO.OF WINDOWS	W1	W2	
FRONT ELEVATION			
NO.OF SETS	10	2	
SILL HEIGHT	0.90METERS ABOVE FLOOR LEVEL	0.90METERS ABOVE FLR. LEVEL	
REMARKS	3" WELDED WIRE MESH W/ 2"X 2" ANGLE BAR FRAME	2 PANEL SLIDING ALUM. FRAME W/ 1/4" THICK CLEAR GLASS	

WINDOW LEGEND

1 : 50

	TIN:	APPROVED BY:	<small> 1. ALL DIMENSIONS ARE IN METERS UNLESS OTHERWISE SPECIFIED. 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE LOCAL GOVERNMENT AND THE NATIONAL BUREAU OF FIRE PROTECTION. 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE NATIONAL BUREAU OF FIRE PROTECTION. 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL NECESSARY PERMITS AND APPROVALS FROM THE NATIONAL BUREAU OF FIRE PROTECTION. </small>	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	<table border="1"> <tr> <td>A7</td> <td>07</td> </tr> <tr> <td colspan="2">SHEET NO.</td> </tr> <tr> <td>07</td> <td>16</td> </tr> <tr> <td colspan="2">PLACE:</td> </tr> </table>	A7	07	SHEET NO.		07	16	PLACE:	
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GENERAL NOTES

1 WORKING DRAWINGS

THIS "GENERAL NOTES FOR STRUCTURAL WORKS " SHALL FORM A PART OF THE STRUCTURAL WORKING DRAWING.

IN THE INTERPRETATION OF THIS DRAWING, INDICATED DIMENSIONS SHALL GOVERN AND DISTANCE OR SIZES SHALL NOT BE SCALED FOR CONSTRUCTION PURPOSES.

IN CASES OF CONFLICT IN DETAILS AND DIMENSIONS BETWEEN THE ARCHITECTURAL AND STRUCTURAL DRAWINGS, REFER TO THE ARCHITECT OR HIS AUTHORIZED REPRESENTATIVE FOR FINAL DECISION.

IN CASES OF CONFLICT BETWEEN THE DRAWINGS, GENERAL NOTES AND SPECIFICATIONS, THE WORKING DRAWING SHALL GOVERN.

IN CASES OF CONFLICT BETWEEN THIS GENERAL NOTES AND SPECIFICATIONS, THE GENERAL NOTES SHALL GOVERN.

DESIGN LOADS

1 DEAD LOADS

THE DESIGN DEAD LOAD UNLESS OTHERWISE SPECIFIED IN THE STRUCTURAL PLANS ARE AS FOLLOWS:

FLOOR MORTAR FINISH	= 10 psf OF FLOOR AREA
FLOOR ARCHITECTURAL FINISH	= 10 psf OF FLOOR AREA
CEILING AND UTILITIES	= 5 psf OF FLOOR AREA
DRYWALL PARTITIONS	= 20 psf OF FLOOR AREA
WATERPROOFING AND CONCRETE COVER	= 25 psf OF FLOOR AREA

THE ARCHITECTURAL AND BUILDING CONTRACTOR SHALL GET THE APPROVAL OF THE STRUCTURAL ENGINEER OF ANY LOADING DIFFERENT FROM OR EXCEEDING THE DESIGN LOADS.

2 LIVE LOADS

THE DESIGN LIVE LOAD UNLESS OTHERWISE SPECIFIED IN THE PLANS ARE AS FOLLOWS :

TYPICAL FLOOR	= 100 psf
MECHANICAL & ELECTRICAL	= 250 psf
ROOF DECK	= 100 psf
ROOF DECK (NON-ACCESSIBLE)	= 30 psf

THESE DESIGN LIVE LOADS SHALL NEVER BE EXCEEDED AT ANY TIME DURING THE LIFE OF THE STRUCTURE WITHOUT THE WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.

IT SHALL BE THE RESPONSIBILITY OF THE OWNER OF THE BUILDING TO GET THE APPROVAL OF THE STRUCTURAL ENGINEER OF ANY LIVE LOAD WHICH WILL BE IMPOSED ON AN AREA OF THE BUILDING DIFFERENT FROM AND OR EXCEEDING THE DESIGN LIVE LOADS SPECIFIED HEREIN.

NO PORTION OF THE BUILDING SHALL BE USED AS TEMPORARY STORAGE OF CONSTRUCTION MATERIALS IN EXCESS OF THE DESIGN LIVE LOADS WITHOUT THE CONSENT OF THE STRUCTURAL ENGINEER.

3 EARTHQUAKE LOADS

THE DESIGN EARTHQUAKE LOADS ARE AS PER NATIONAL STRUCTURAL CODE OF THE PHILIPPINES 2001 EDITION

4 WIND LOADS

THE DESIGN WIND LOADS ARE AS PER NATIONAL STRUCTURAL CODE OF THE PHILIPPINES 2001 EDITION

5 EQUIPMENT LOADING

EQUIPMENT NOT INDICATED IN THE PLANS SHALL NOT BE INSTALLED WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER. THE MANUFACTURER SHALL SUBMIT EQUIPMENT DATA SPECIFYING THE WEIGHT, AND ITS REACTION AT THE BASE, AND ITS VIBRATION CHARACTERISTICS.

REINFORCED CONCRETE CONSTRUCTION

1 CEMENT SHALL CONFORM TO PORTLAND CEMENT ASTM C150.

2 CONCRETE AGGREGATE SHALL CONFORM TO ASTM C33 EXCEPT THAT AGGREGATES FAILING TO MEET THOSE SPECIFICATION BUT WHICH HAVE PRODUCED CONCRETE OF ADEQUATE STRENGTH AND DURABILITY MAYBE USED SUBJECT TO THE APPROVAL OF THE ENGINEER.

3 WATER USED IN MIXING CONCRETE SHALL BE CLEAN AND FREE FROM INJURIOUS AMOUNT OF OILS, ACIDS, ALKALIS, SALTS, ORGANIC MATERIALS OR OTHER SUBSTANCES DELETERIOUS TO CONCRETE AND STEEL.

4 REINFORCING BARS SHALL CONFORM TO ASTM A615.

5 ADMIXTURES TO BE USED IN CONCRETE SHALL BE SUBJECT TO PRIOR APPROVAL BY THE STRUCTURAL ENGINEER.

6 CEMENT AND AGGREGATES SHALL BE STORED IN SUCH A MANNER AS TO PREVENT THEIR DETERIORATION OR THE INTRUSION OF FOREIGN MATTER.

7 CONCRETE CYLINDER SAMPLES FOR STRENGTH TESTS OF EACH CLASS OR CONCRETE SHALL BE TAKEN NOT LESS THAN TWICE A DAY OR NOR LESS THAN ONCE FOR EACH 50 CU. m. OF CONCRETE PLACE.

8 THE CYLINDER SAMPLES FOR STRENGTH TESTS SHALL BE TAKEN CURED AND TESTED IN ACCORDANCE WITH ASTM C172, ASTM C31, ASTM C39.

9 ACCEPTANCE OF CONCRETE. CONCRETE POURED WILL BE CONSIDERED SATISFACTORY IF THE AVERAGES OF ALL SETS OF THREE CONSECUTIVE STRENGTH TEST RESULTS EQUAL OR EXCEED THE REQUIRED f_c' AND NO INDIVIDUAL STRENGTH TEST RESULTS FALL BELOW THE REQUIRED f_c' BY MORE THAN 500 psi.

10 CORE TESTS AND LOAD TESTS. IF INDIVIDUAL TESTS OF LABORATORY CURED CYLINDER SAMPLES PRODUCED STRENGTH MORE THAN 500 psi BELOW f_c' CORE TESTS MAY BE RESORTED SUBJECT TO THE APPROVAL BY THE STRUCTURAL ENGINEER.

11 MIXING OF CONCRETE. ALL CONCRETE SHALL BE MIXED UNTIL THERE IS UNIFORM DISTRIBUTION OF THE MATERIALS AND SHALL BE DISCHARGED COMPLETELY BEFORE THE MIXER IS RECHARGED.

12 CONVEYING OF CONCRETE. CONCRETE SHALL BE CONVEYED FROM THE MIXER TO THE PLACE OF FINAL DEPOSIT BY METHODS WHICH WILL PREVENT THE SEPARATION OR LOSS OF MATERIALS.

13 DEPOSITING OF CONCRETE. CONCRETE SHALL BE DEPOSITED AS NEARLY AS PRACTICABLE IN ITS FINAL POSITION TO AVOID SEGREGATION DUE TO REHANDLING OR FLOWING.

14 CURING. CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR AT LEAST 7 DAYS AFTER PLACING. WET BURLAP MAYBE LAYED OVER THE SLAB CONSTANTLY APPLIED WITH WATER.

CONCRETE MIXES

UNLESS OTHERWISE INDICATED IN THE DRAWINGS, THE MINIMUM 28-DAYS CYLINDER COMPRESSIVE STRENGTH SHALL BE AS FOLLOWS:

A. FOUNDATION, CONCRETE WALLS, COLUMNS, UNDERGROUND TANKS, SUSPENDED BEAMS AND SLABS
 $f_c' = 4,000$ psi

B. SLAB ON GRADE
 $f_c' = 3,000$ psi

FOOTINGS

- THE ALLOWABLE SOIL BEARING PRESSURE IS 6,000 PSF
- EXCAVATION FOR FOOTINGS SHALL BE CARRIED TO A DEPTH AS SPECIFIED IN THE PLANS.
- EXISTING UNDERGROUND PIPES, TUNNELS, ETC. SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER FOR EVALUATION.
- ALL EARTH FILL SUPPORTING BASEMENT SLABS FOR FLOORING, PARKING, SIDEWALK, ETC. SHALL BE COMPACTED TO 95% PROCTOR UNLESS OTHERWISE SPECIFIED BY THE STRUCTURAL ENGINEER.

REINFORCED CONCRETE BEAMS

- UNLESS OTHERWISE NOTED IN THE PLANS OR SPECIFICATIONS CAMBER ALL RC BEAMS AT LEAST 6 mm FOR EVERY 4.5 m. OF SPAN EXCEPT CANTILEVERS WHICH SHALL BE 18 mm FOR EVERY 3.0 m. SPAN.
- IF THERE ARE TWO OR MORE LAYERS OF REINFORCING BARS, USE 25# SEPARATORS SPACED AT 1200 mm O.C.
- IF BEAMS REINFORCING BARS ENDS IN A WALL, THE CLEAR DISTANCE FROM THE BAR TO THE FARTHER FACE OF THE WALL SHALL BE NOT LESS THAN 5 DIAMETERS. THE REINFORCING BARS SHALL TERMINATE ON A STANDARD OF 90° HOOK.
- SPLICES FOR TOP BARS SHALL BE LOCATED AT MID-SPAN AND BOTTOM BARS SHALL BE SPLICED AT THE DISTANCE OF TWICE THE MEMBER DEPTH FROM THE FACE OF THE COLUMN. LENGTH OF SPLICE SHALL BE HELD TO PROVIDE A MINIMUM OF TWO STIRRUPS TIES AT SPLICE. SPLICES INDICATE MAYBE STAGGERED A DISTANCE OF AT LEAST 40 BARS DIAMETER A MAXIMUM OF 50% OF TOP OR BOTTOM BAR MAY BE SPLICED AT ANY ONE LOCATION.

REINFORCED CONCRETE SLABS

- UNLESS OTHERWISE NOTED IN PLANS OR SPECIFICATIONS, CAMBER ALL R.C. SLABS 3 mm PER 3.0 m. OF SHORTER SPAN.
- IF SLABS ARE REINFORCED BOTH WAYS, THE SHORTER SPAN REBAR SHALL BE THE BOTTOM BARS.
- CONCRETE COVERING SHALL BE 19 mm FOR TOP AND FOR BOTTOM BARS.
- UNLESS OTHERWISE SPECIFIED BY THE ENGINEER BAR CHAIRS SHALL BE PROVIDED AT LEAST 0.90 m. EACH WAY TO SUPPORT TOP AND BOTTOM SLAB SEPARATELY.

REINFORCED CONCRETE COLUMNS

- CONSTRUCTION JOINTS IN COLUMNS SHALL BE LOCATED AT MIDHEIGHT OF COLUMN (BETWEEN FLOOR LINE.)
- ALL TIES SHALL BE FASTENED TO COLUMN VERTICAL REINFORCEMENT BY MEANS OF WIRES AT ALL INTERSECTION POINTS OF TIES AND COLUMN REBARS.
- NOT MORE THAN ONE-THIRD (1/3) OF THE TOTAL NUMBER OF BARS SHALL BE SPLICED AT THE SAME LEVEL. THE LAP SPLICE SHALL BE 1.2 L_d WHERE L_d IS THE DEVELOPMENT LENGTH AS TABULATED IN THE STANDARD DETAILS. SPLICES SHALL BE STAGGERED A DISTANCE OF AT LEAST L_d .
- CONFINEMENT TIES SHALL BE PROVIDED ON ALL COLUMNS AT BEAM COLUMN INTERSECTIONS AS SHOWN IN STANDARD DETAILS.

REINFORCED CONCRETE WALLS

- UNLESS OTHERWISE INDICATED IN THE PLANS THE R.C. WALL HORIZONTAL BARS SHALL ENCLOSE THE VERTICAL BARS.
- CONCRETE CLEARANCES FOR REINFORCING BARS - (SEE ITEM 12) FOR 100 mm WALL OR LESS REINFORCING BARS SHALL BE AT THE CENTER.
- CARRY VERTICAL BARS 0.60 m. ABOVE FLOOR LEVEL TO PROVIDE FOR SPLICES WHEN NECESSARY. ELSEWHERE STOP AT 50 mm BELOW THE TOP OF THE SLAB. THE BAR SHALL TERMINATE ON STANDARD 90° DEGREE HOOK.
- HORIZONTAL AND VERTICAL BARS CONFORMING TO ASTM A615 SHALL BE SPLICED BY LAPPING AND WIRED WITH NO.16 G.I. WIRE AND CONFORMING SECTION 1 PROVIDED THAT SPLICES IN ADJACENT BARS ARE STAGGERED AT LEAST 1.50 m. O.C. NOT MORE THAN ONE-HALF (1/2) OF THE TOTAL NUMBER OF BARS SHALL BE SPLICED AT THE SAME LINE.
- ALL OPENINGS ON WALL OR SLABS LESS THAN 200 mm. THICK SHALL BE REINFORCED AS SHOWN IN THE STANDARD DETAILS.

CHB WALLS

- UNLESS OTHERWISE INDICATED THE VERTICAL AND HORIZONTAL REINFORCEMENTS FOR C.H.B. SHALL BE 10 mm. AT 600 O.C. FOR 0.15 m. THICK BLOCKS. MINIMUM LAP SPLICE SHALL BE 300 mm. (REFER TO ARCHITECTURAL FLOOR PLAN FOR LOCATION OF C.H.B. WALLS).
- LINTEL BEAMS TO BE USED SHALL BE (T X 0.40 m.) REINFORCED BY 4-16 mm WITH 10 mm AT 250 TIES WHERE "T" IS THE CHB WALL THICKNESS. (UNLESS OTHERWISE SPECIFIED ON DETAILS).
- LINTEL BEAMS SHALL BE PROVIDED AT TOP OF CHB WALL OPENINGS IT SHALL BE EXTENDED AT LEAST 0.20 m. BEYOND THE OPENINGS.
- FOR HIGH WALLS, LINTEL BEAMS INTERVAL SHALL BE PROVIDED AT 3.0 m. 3.0 m. VERTICAL
- FOR LONG WALLS, LINTEL BEAMS ACTING AS COLUMN SHALL BE PROVIDED EVERY 6 METERS
- WHERE CHB WALLS ADJOIN R.C. COLUMNS AND BEAMS, PROVIDE DOWELS ON R.C. COLUMN AND BEAMS PRIOR TO POURING TO MATCH CHB WALL REINFORCEMENT.
- WHERE THE TOP OF A CHB WALL ADJOIN A BEAM OR SLAB, REBARS AS REQUIRED IN 10.6 SHALL BE RETAINED FOR STABILITY. (SEE STANDARD DETAIL)
- WHERE SIDES OF A CHB WALL ADJOIN A COLUMN OR RC WALL - SEE CONTROL JOINT DETAIL AT STANDARD DETAILS. REBARS AS REQUIRED IN 10.6 SHALL BE RETAINED FOR STABILITY.
- FOR HIGH WALLS, LINTEL BEAMS INTERVAL SHALL BE PROVIDED AT 3.0 m.
- NO CHIPPING OFF OF CONCRETE COLUMNS AND BEAMS ARE ALLOWED UNLESS OTHERWISE PERMITTED BY THE ENGINEER WHERE COLUMNS AND BEAMS HAVE BEEN POURED WITHOUT DOWELS FOR THE CHB WALL PROVIDE 12# DRILLED DOWELS @ 600mm O.C. WITH EPOXY GROUT

STRUCTURAL TOLERANCES

UNLESS OTHERWISE SPECIFIED BY THE ENGINEER, THE FOLLOWING ARE THE ACCEPTABLE STRUCTURAL TOLERANCES FOR CAST-IN-PLACE CONCRETE CONSTRUCTION.

- | | | |
|--|---|---------------------|
| A. CROSS SECTIONAL DIMENSIONS AND LOCATION TO REINFORCEMENT. | | |
| DIMENSION LESS THAN 200 mm TO 600 mm | = | ± 6 mm |
| OVER 600 mm | = | ± 9 mm |
| B. MEMBER LENGTH OR HEIGHT | = | ± 12 mm |
| (MAXIMUM LIMITATION = 12 mm) | | |
| C. DEVIATION FROM STRAIGHT LINE (SWEEP AND OR PLUMBNESS) | = | 6 mm PER 3.0 METERS |
| D. LOCATION OF BAR CUT-OFFS OR BENDS | = | ± 50 mm |

CONCRETE PROTECTION FOR REINFORCEMENT

THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING BARS. FOR BAR BUNDLES THE MINIMUM COVER SHALL EQUAL THE EQUIVALENT DIAMETER OF THE BUNDLE. BUT NEED NOT BE MORE THAN 50 mm ON THE TABULATED MINIMUM WHICHEVER IS GREATER.

- CAST-IN-PLACE CONCRETE (REINFORCED CONCRETE CONSTRUCTION.)

A. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	MINIMUM COVER	75 mm
B. EXPOSED TO EARTH OR WEATHER 20# AND LARGER		50 mm
16# AND SMALLER		40 mm
C. NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND SLABS, WALL AND JOISTS		
16# AND SMALLER		20 mm
BEAMS, GIRDERS AND COLUMNS PRINCIPAL REINFORCEMENT, TIES STIRRUPS OR SPIRALS		40 mm

REINFORCING BARS

- ALL MAIN REINFORCING BARS SHALL BE DEFORMED CONFORMING TO ASTM A615-68
- UNLESS OTHERWISE NOTED IN THE PLANS THE MINIMUM YIELD STRENGTH OF THE MAIN REINFORCING BARS SHALL BE AS FOLLOWS:

12# mm AND SMALLER	$f_y = 40,000$ psi
16# mm AND LARGER	$f_y = 60,000$ psi

- IN STANDARD DETAILS.

STANDARD HOOK

- A STANDARD HOOK FOR REBARS IF REQUIRED SHALL BE EITHER OF THE FOLLOWING:
 - A SEMI-CIRCULAR TURN PLUS AN EXTENSION OF AT LEAST 4 DIAMETER BUT NOT LESS THAN 62 mm AT THE FREE END OF THE BAR.
 - A 90° TURN PLUS EXTENSION OF AT LEAST 12 BAR DIAMETERS AT THE FREE END OF THE BAR.

=	6 BAR DIAMETERS
=	8 BAR DIAMETERS

- MINIMUM DIAMETERS OF BEND MEASURED ON THE INSIDE OF THE BAR TO THE FREE END OF THE BAR.

10 mm TO 25 mm
28 mm TO 36 mm

CONSTRUCTION JOINTS

- CONSTRUCTION JOINTS SHALL BE LOCATED NEAR THE MIDDLE OF SPAN OF SLABS, BEAMS OR GIRDER.
- AT BEAM-GIRDER INTERSECTION, THE CONSTRUCTION JOINT ON THE GIRDER SHALL BE OFFSET A DISTANCE EQUAL TO TWICE THE WIDTH OF THE BEAM. DIAGONAL BARS SHALL BE PROVIDED TO RESIST 100 % SHEAR AT THE CONSTRUCTION JOINT.

THE TOTAL LENGTH OF THE DIAGONAL BAR SHALL BE AT LEAST TWICE THE DEVELOPMENT LENGTH REQUIRED FOR THE BAR. THE ENDS OF THE BAR SHALL BE HOOKED.

- WHERE A JOINT IS TO BE MADE THE SURFACE OF THE CONCRETE SHALL BE THOROUGHLY CLEANED AND ALL LAITANCE AND STANDING WATER REMOVED. VERTICAL COATED WITH NEAT CEMENT GROUT IMMEDIATELY BEFORE PLACING OF NEW CONCRETE.

ELECTRICAL CONDUITS

- ELECTRICAL CONDUITS MAY BE PLACED BETWEEN THE TOP AND BOTTOM REINFORCEMENT IN SLABS AND R.C. WALLS.
- THE SPACING OF THESE CONDUITS SHALL BE NOT CLOSER THAN 3 BARS DIAMETER

PIPES EMBEDDED IN CONCRETE

- PIPES TO BE EMBEDDED ON COLUMNS SHALL BE SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER.
- NO VERTICAL PIPES ARE ALLOWED TO PUNCH THROUGH A BEAM OR GIRDER.
- HORIZONTAL PIPES MAY PUNCH THROUGH A BEAM OR GIRDER SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER.

THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER THE ACTUAL LOCATION WHERE PIPES CROSS A BEAM OR GIRDER, WHERE PERMITTED BY THE ENGINEER PROVIDE AT LEAST 16 mm (600 mm LONG) U-BARS TO ENCLOSE THE PIPE AT THE POINT OF INTERSECTION AT EACH FACE OF THE BEAM.

- PIPES OF ALUMINUM SHALL NOT BE EMBEDDED IN CONCRETE.
- CONCRETE COVER FOR PIPES SHALL BE AT LEAST 38 mm FOR CONCRETE SURFACE EXPOSED TO THE WEATHER OR IN CONTACT WITH THE GROUND 18 mm FOR CONCRETE SURFACE NOT EXPOSED TO THE GROUND OR WEATHER.


REMOVAL OF FORMS AND SHORES

- FORMS SHALL BE REMOVED IN SUCH MANNER AS TO ENSURE COMPLETE SAFETY OF THE STRUCTURE.
- UNLESS OTHERWISE SPECIFIED BY THE STRUCTURAL ENGINEER THE FOLLOWING SHALL BE THE BASIS OF FORMS AND SHORINGS REMOVAL FOR REINFORCED CONCRETE (R.C.) CONSTRUCTION.

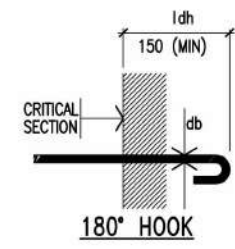
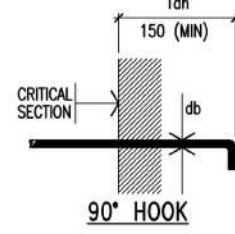
BEAM SIDE FORMS	2 DAYS	} AGE OF CONCRETE
COLUMN SIDE FORMS	2 DAYS	
SLAB BOTTOM FORMS	5 DAYS	
BEAM SHORING	8 DAYS	
SLAB SHORING	8 DAYS	

STRUCTURAL SPECIFICATIONS

1 : 200

	TIN:	APPROVED BY:	PROJECT TITLE: CONSTRUCTION OF VSU POWER PLANT BUILDING	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT: STRUCTURAL SPECIFICATIONS	DESIGNED BY:	<table border="1"> <tr> <td>S6</td> <td>01</td> </tr> <tr> <td colspan="2">SHEET NO.</td> </tr> <tr> <td>08</td> <td>16</td> </tr> </table>	S6	01	SHEET NO.		08	16
	S6	01													
	SHEET NO.														
	08	16													
	PRC:			ENGR. MARIO LILIO P. VALENZONA	DR. DANIEL LESLIE S. TAN	DR. EDGARDO E. TULIN									
PTR:															
DATE:															
PLACE:	STRUCTURAL ENGINEER		LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A	DIRECTOR, PPO	VP OF ADMINISTRATIVE AND FINANCE	VSU PRESIDENT									

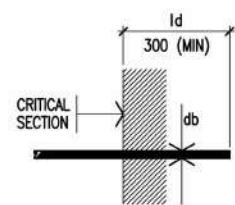
BAR DIAMETER (mm)	BAR GRADE (MPa)	BASIC DEVELOPMENT LENGTH, l_{db} (mm)	
		$f'_c=20.7$ MPa (3,000psi)	$f'_c=27.6$ MPa (4,000psi)
10	275.80	150	150
	413.70	219	190
12	275.80	175	152
	413.70	263	228
16	275.80	234	202
	413.70	351	304
20	275.80	292	253
	413.70	438	379
25	275.80	365	316
	413.70	548	474
28	275.80	409	354
	413.70	613	531
32	275.80	467	405
	413.70	701	607
36	275.80	526	455
	413.70	789	683



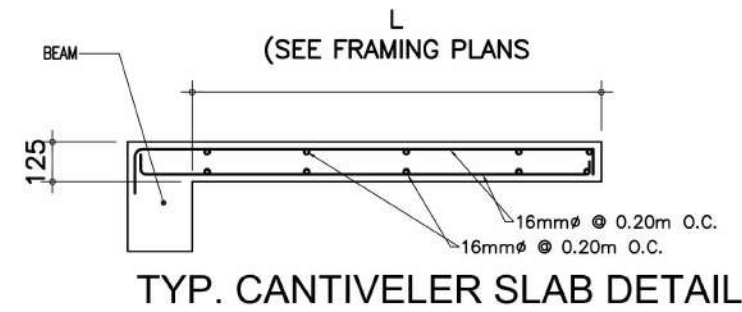
- DEVELOPMENT LENGTH, l_{db} SHALL BE AS FOLLOWS:
- FOR BARS W/ SIDE COVER (NORMAL TO PLANE HOOK) NOT LESS THAN 65mm AND FOR 90° HOOK, COVER ON BAR EXTENSION BEYOND HOOK NOT LESS THAN 50mm, USE 0.7 l_{db} .
 - FOR BARS W/ HOOK ENCLOSED VERTICALLY OR OR HORIZONTALLY W/IN TIES OR STIRRUP TIES SPACED ALONG THE FULL DEVELOPMENT LENGTH l_{db} NOT GREATER THAN $3d_b$, USE 0.8 l_{db} .
 - FOR BARS NOT INCLUDED IN (a) & (b), USE 1.0 l_{db} .

DEVELOPMENT LENGTH OF HOOKED BARS

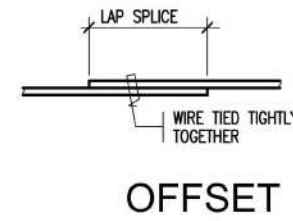
BAR DIAMETER (mm)	BAR GRADE (MPa)	MINIMUM DEVELOPMENT LENGTH l_d (mm)			
		$f'_c=20.7$ MPa (3,000psi)		$f'_c=27.6$ MPa (4,000psi)	
		TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
10	275.80	300	300	300	300
	413.70	427	329	370	300
12	275.80	342	300	300	300
	413.70	513	394	444	342
16	275.80	456	351	395	304
	413.70	684	526	592	455
20	275.80	570	438	493	379
	413.70	854	657	740	569
25	275.80	734	565	636	489
	413.70	1101	847	953	733
28	275.80	921	708	797	613
	413.70	1381	1062	1196	920
32	275.80	1202	925	1041	801
	413.70	1804	1387	1562	1202
36	275.80	1522	1171	1318	1014
	413.70	2283	1756	1977	1521



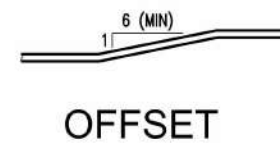
DEVELOPMENT LENGTH OF STRAIGHT BARS



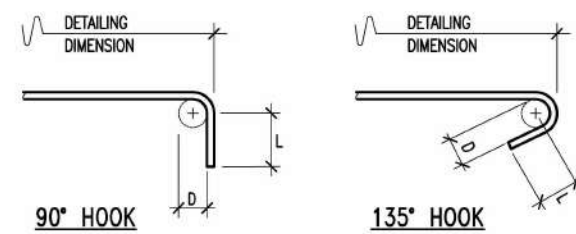
TYP. CANTILEVER SLAB DETAIL



OFFSET



OFFSET

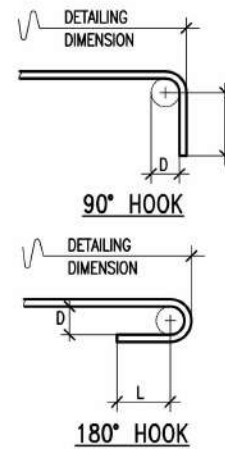


BAR DIAMETER (mm)	D (mm)	L (mm)		
		TYPE I		
		90° HOOK	135° HOOK	135° HOOK
10	40	60	60	75
12	48	72	72	75

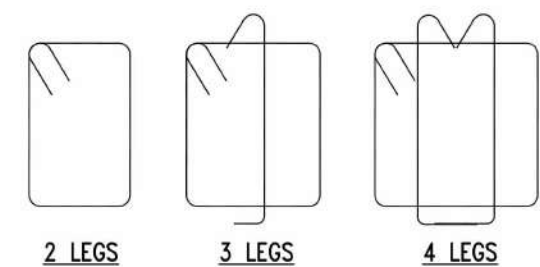
NOTE: TYPE I FOR GENERAL USE
TYPE II FOR SEISMIC USE (FOR ALL COLUMNS AND ALL BEAMS CONNECTED TO COLUMNS)

STIRRUPS AND TIE HOOK DIMENSIONS

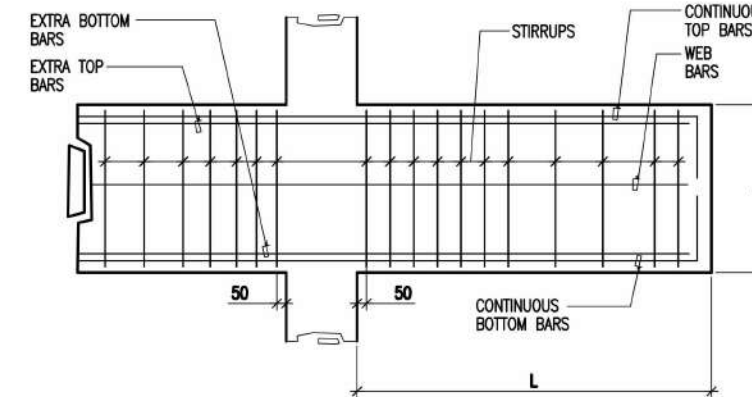
BAR DIAMETER (mm)	D (mm)	L (mm)	
		180° HOOK	90° HOOK
10	60	65	120
12	72	65	145
16	96	65	192
20	120	80	240
25	150	100	305
28	225	115	335
32	255	130	385
36	288	144	432



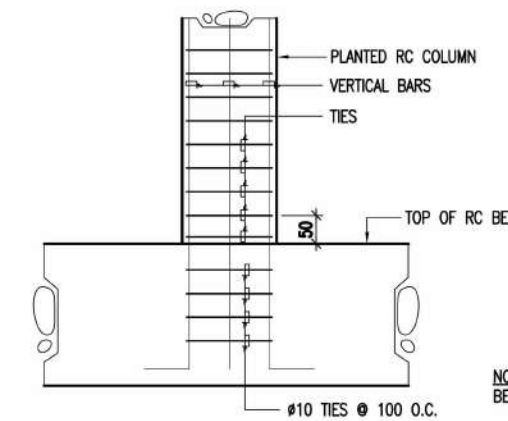
STANDARD HOOK DIMENSIONS



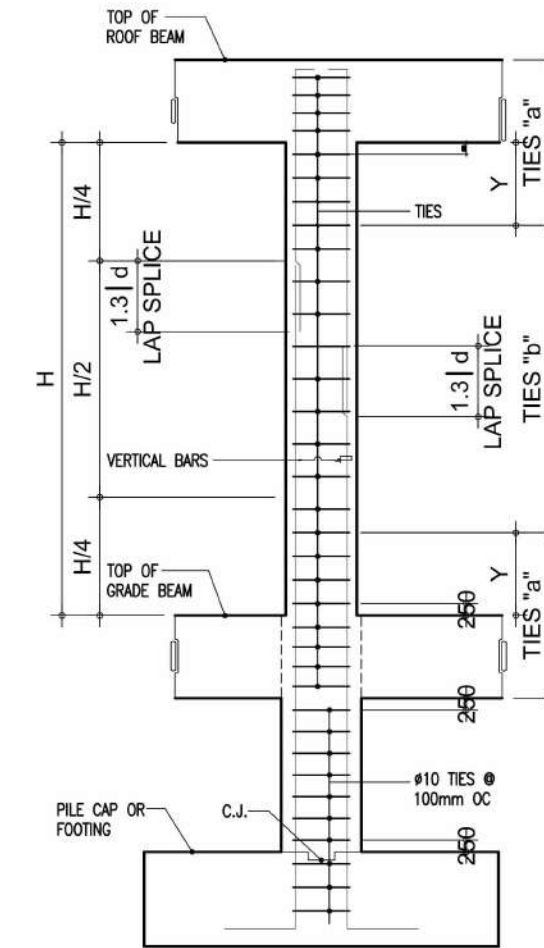
TYPICAL STIRRUPS DETAIL



TYPICAL CANTILEVER BEAM DETAIL



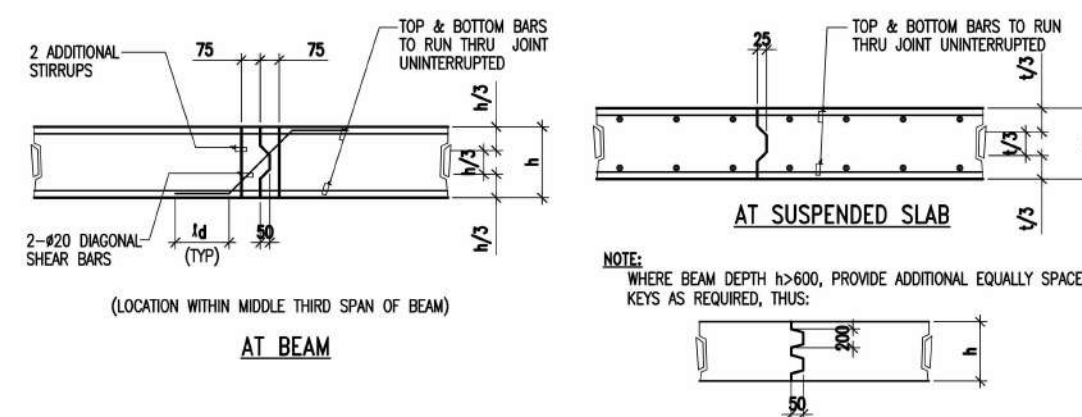
TYPICAL PLANTED RC COLUMN DETAIL



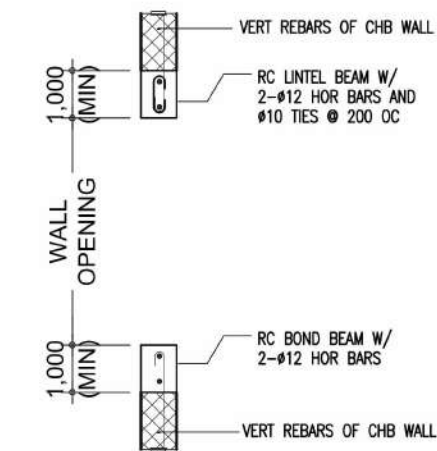
C.J. = CONSTRUCTION JOINT

- NOTES:
- Y-MAX OF THE FF:
 - H/6
 - 450 MM
 - MAXIMUM COLUMN DIMENSION
 - SPLICES ARE PERMITTED ONLY WITHIN THE CENTER HALF OF COLUMN HEIGHT (H)
 - STAGGER BAR SPLICES BY 600 MM OR MORE
 - PROMOTE TIES @ 100 MM O.C. (MAX) OVER THE FULL LAP SPLICE LENGTH
 - SPECIAL TIES @ BEAM-COLUMN JOINT TO CONFORM TO THE SAME CONFIGURATION OF TIES AS INDICATED IN THE SCHEDULE OF COLUMNS

TYPICAL RC COLUMN ELEVATION



TYPICAL CONSTRUCTION JOINT DETAILS

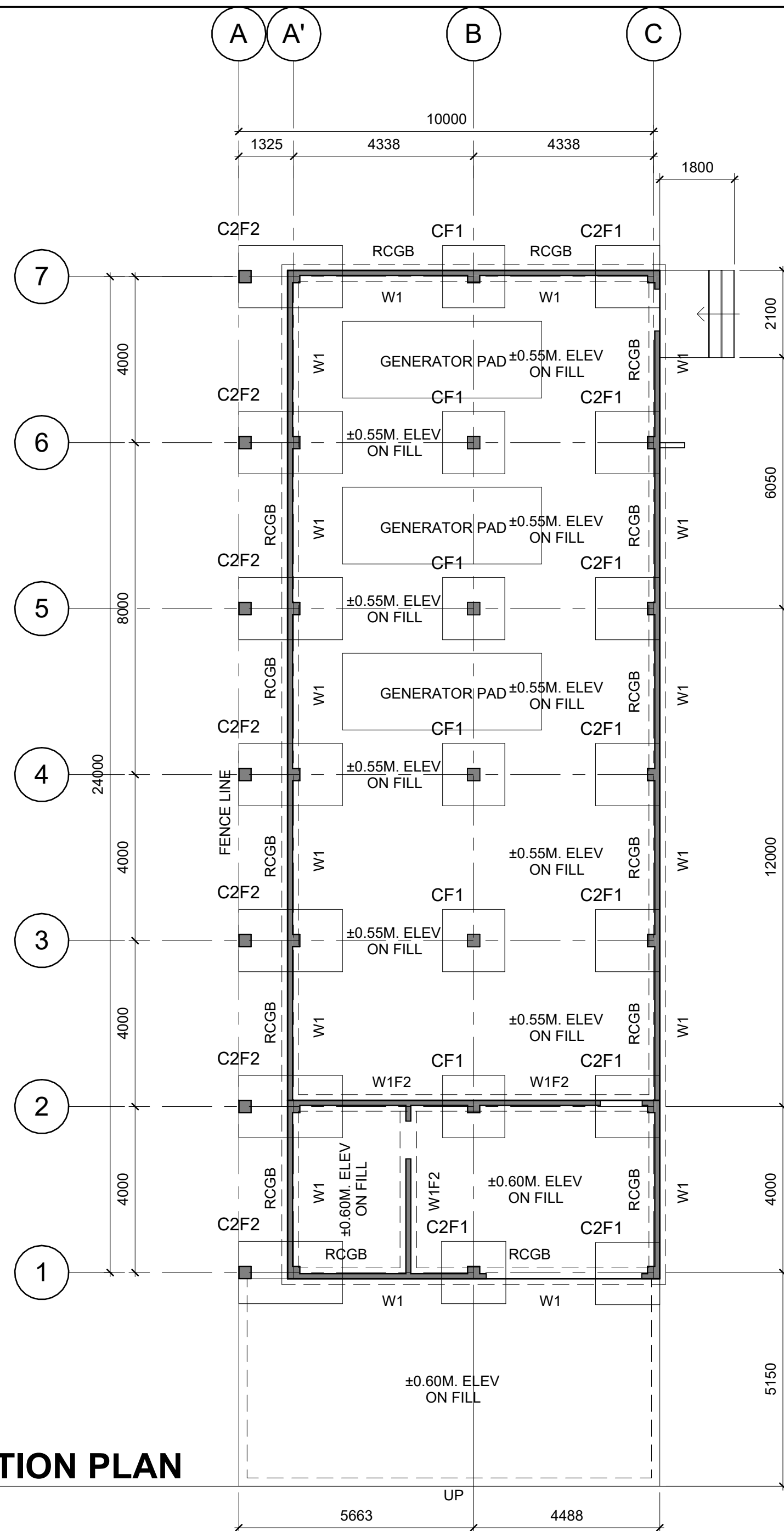


TYPICAL WALL OPENING DETAIL

STRUCTURAL DETAILS

1 : 40

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	S6	02													
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09	16														
PRC:			CONSTRUCTION OF VSU POWER PLANT BUILDING	ENGR. MARIO LILIO P. VALENZONA	DR. DANIEL LESLIE S. TAN	DR. EDGARDO E. TULIN	STRUCTURAL DETAILS	CADD BY:							
PTR:			LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A	DIRECTOR, PPO	VP OF ADMINISTRATIVE AND FINANCE	VSU PRESIDENT		STARTED:							
DATE:								FINISHED:							
PLACE:	STRUCTURAL ENGINEER							PLACE:							



1 FOUNDATION PLAN
1 : 100

SCHEDULE OF R.C. COLUMNS

FLOOR LEVEL	C1				C2			
ROOF LEVEL	COLUMN SIZE 250mmx300mm	MAIN BARS		8-16MM#	COLUMN SIZE 300mmx300mm	MAIN BARS		12-16MM#
		TIES				TIES		
		BELOW BEAM SOFFIT	NO. OF TIE, DIA. & SPACING	TYPE		BELOW BEAM SOFFIT	NO. OF TIE, DIA. & SPACING	TYPE
		MIDLEVEL	NO. OF TIE, DIA. & SPACING	TYPE		MIDLEVEL	NO. OF TIE, DIA. & SPACING	TYPE
GROUND FLOOR LEVEL	COLUMN SIZE 300mmx300mm	MAIN BARS		8-16MM#	COLUMN SIZE 300mmx300mm	MAIN BARS		12-16MM#
		TIES				TIES		
		BELOW BEAM SOFFIT	NO. OF TIE, DIA. & SPACING	TYPE		BELOW BEAM SOFFIT	NO. OF TIE, DIA. & SPACING	TYPE
		MIDLEVEL	NO. OF TIE, DIA. & SPACING	TYPE		MIDLEVEL	NO. OF TIE, DIA. & SPACING	TYPE
BELOW GROUND LEVEL	COLUMN SIZE 300mmx300mm	MAIN BARS		8-16MM#	COLUMN SIZE 300mmx300mm	MAIN BARS		12-16MM#
		TIES				TIES		
		BELOW BEAM SOFFIT	NO. OF TIE, DIA. & SPACING	TYPE		BELOW BEAM SOFFIT	NO. OF TIE, DIA. & SPACING	TYPE
		MIDLEVEL	NO. OF TIE, DIA. & SPACING	TYPE		MIDLEVEL	NO. OF TIE, DIA. & SPACING	TYPE

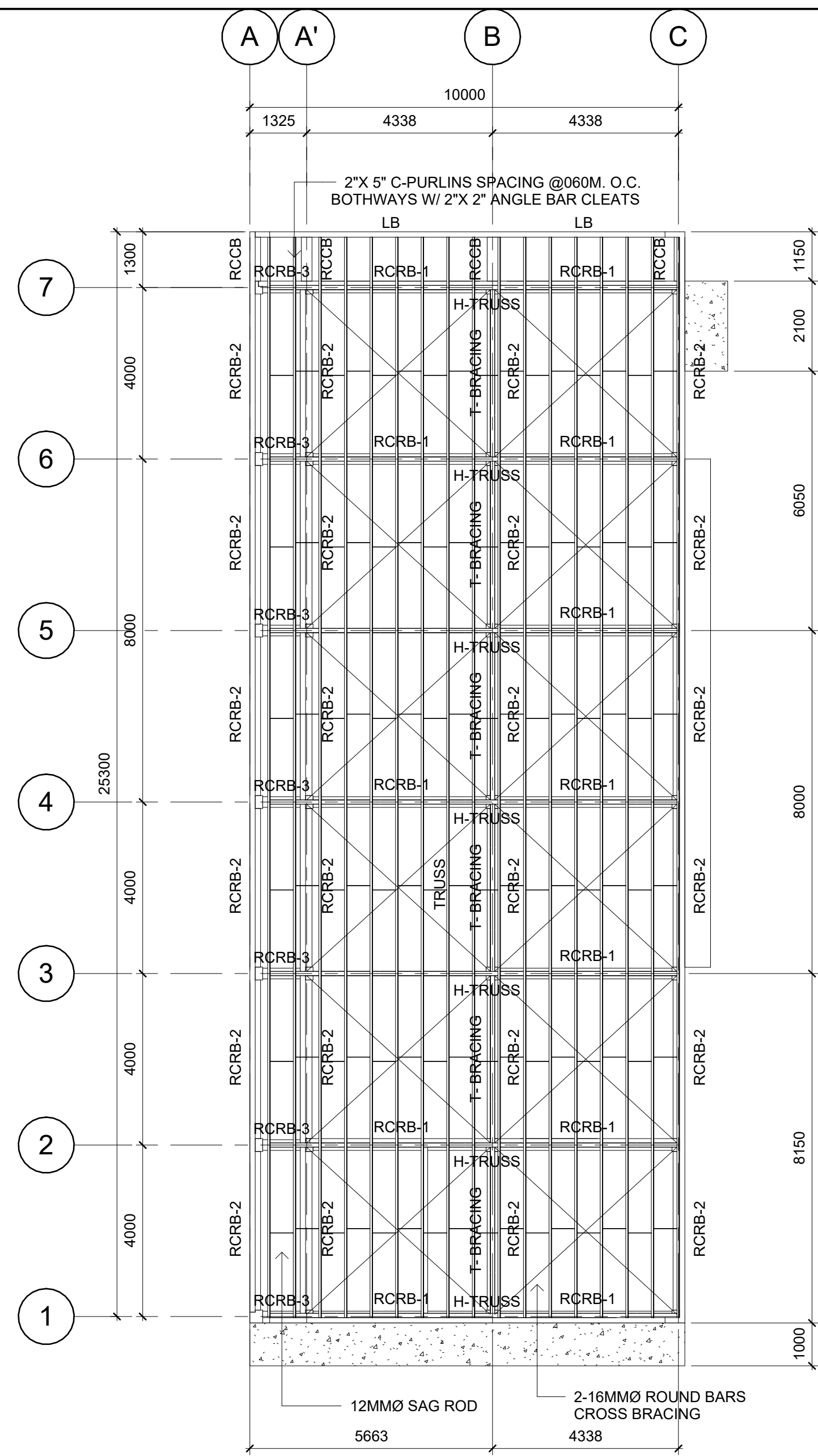
SCHEDULE OF R.C. FOOTINGS

F.MARK	SIZE (mm)	THICKNESS (mm)	REINFORCEMENTS			REMARKS
			LOCATION	X-DIRECTION	Y-DIRECTION	
F1	1500X1500	250	TOP			
			BOTTOM	12-16Φ	12-16Φ	
F2	1500X2500	300	TOP			
			BOTTOM	14-16Φ	14-16Φ	

SCHEDULE OF R.C. GROUND BEAM

B.MARK	SECTION (mm)	REINFORCEMENTS					Side Bar	STIRRUPS		
		LOCATION	TYPE	CONT.	MIDSPAN	DISCONT		NO. OF LEGS & BAR DIA	NUMBER & SPACING NEAR SUPPORT (mm)	SPACING @ MIDSPAN (mm)
RCGB	350X300	TOP	S	2-12Φ	2-12Φ	2-12Φ	1-12Φ EF	2L - 10Φ	4-50, 6-100	200
			E							
		BOTTOM	S	2-12Φ	2-12Φ	2-12Φ				
			E							

SCHEDULE OF BEAM, COLUMNS & FOOTINGS
1 : 70

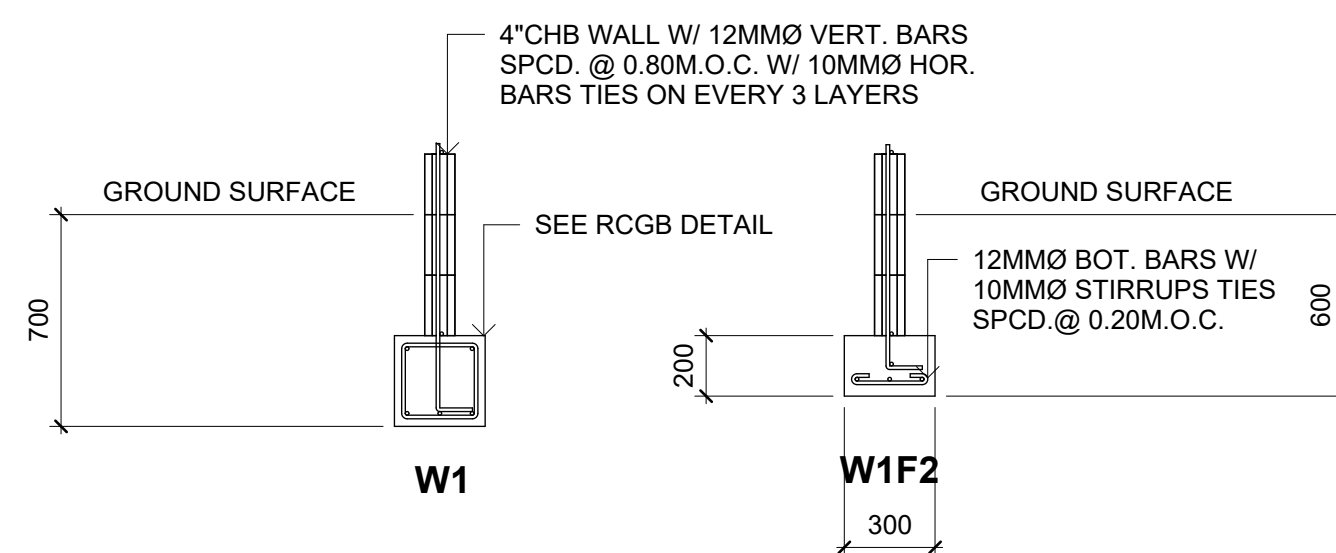
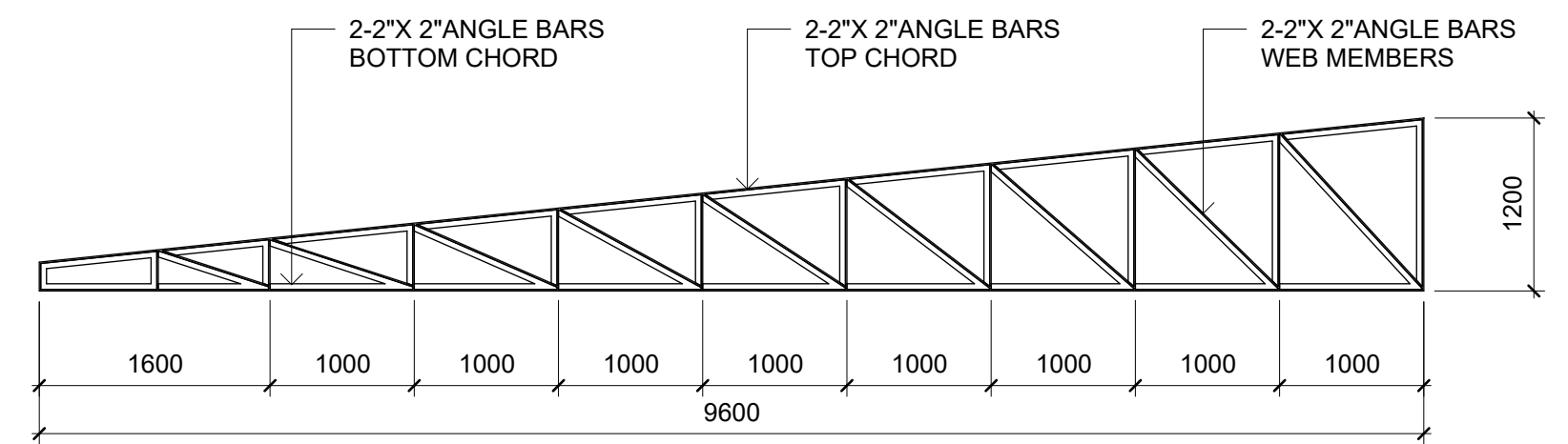


SCHEDULE OF CANTILIVER, LINTEL & ROOF BEAM

BEAM MARK	SECTION (mm)	REINFORCEMENTS					Side Bar	STIRRUPS			REMARKS
		LOCATION	TYPE	CONT.	MIDSPAN	DISCONT		NO. OF LEGS & BAR DIA	NUMBER & SPACING NEAR SUPPORT (mm)	SPACING @ MIDSPAN (mm)	
RCRB-1	450X250	TOP	S	2-16Φ	2-16Φ	2-16Φ	1-16Φ EF	2L - 10Φ	4-50, 6-100	200	USE #16 TIE WIRE AND SPECIFY THE STIRRUPS SPACING
			E		1-16Φ						
RCRB-2	400X250	TOP	S	2-16Φ	2-16Φ	2-16Φ	1-16Φ EF	2L - 10Φ	4-50, 6-100	200	USE #16 TIE WIRE AND SPECIFY THE STIRRUPS SPACING
			E		1-16Φ						
RCRB-3	350X250	TOP	S	2-12Φ	2-12Φ	2-12Φ	1-12Φ EF	2L - 10Φ	4-50, 6-100	200	USE #16 TIE WIRE AND SPECIFY THE STIRRUPS SPACING
			E								
RCCB	400X250	TOP	S	2-16Φ	2-16Φ	2-16Φ	1-16Φ EF	2L - 10Φ	4-50, 6-100	200	USE #16 TIE WIRE AND SPECIFY THE STIRRUPS SPACING
			E								
LB	300X200	TOP	S	2-12Φ	2-12Φ	2-12Φ	1-12Φ EF	2L - 10Φ	3-50, 5-100	200	USE #16 TIE WIRE AND SPECIFY THE STIRRUPS SPACING
			E								

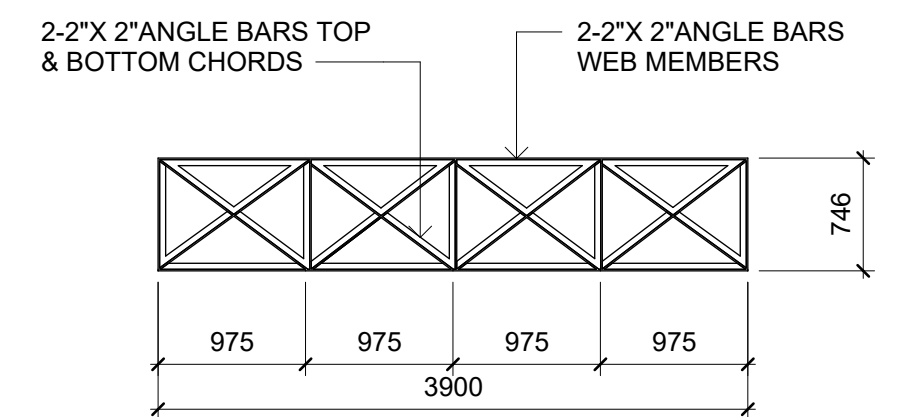
SCHED. OF R.C. CANTILIVER, LINTEL & ROOF BEAMS

1 : 140



DETAIL OF WALL & FOOTINGS

1 : 25



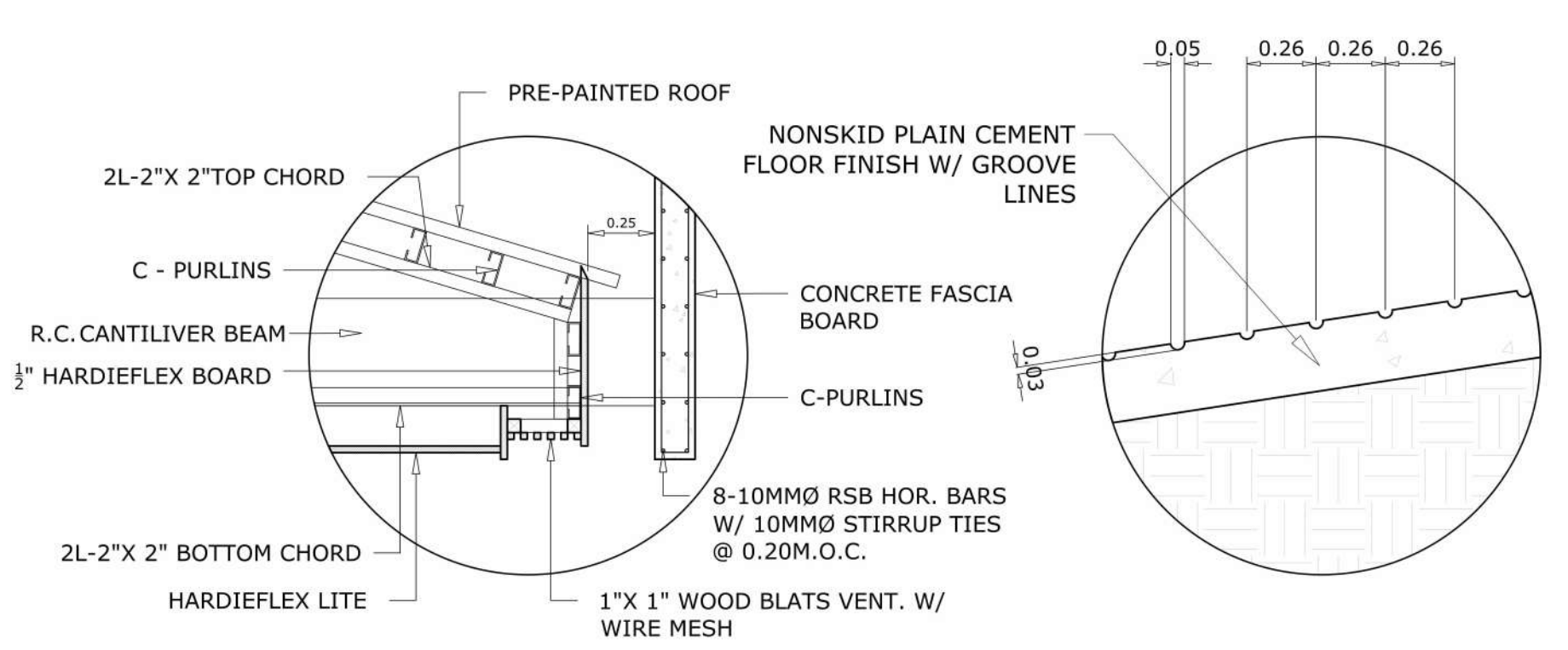
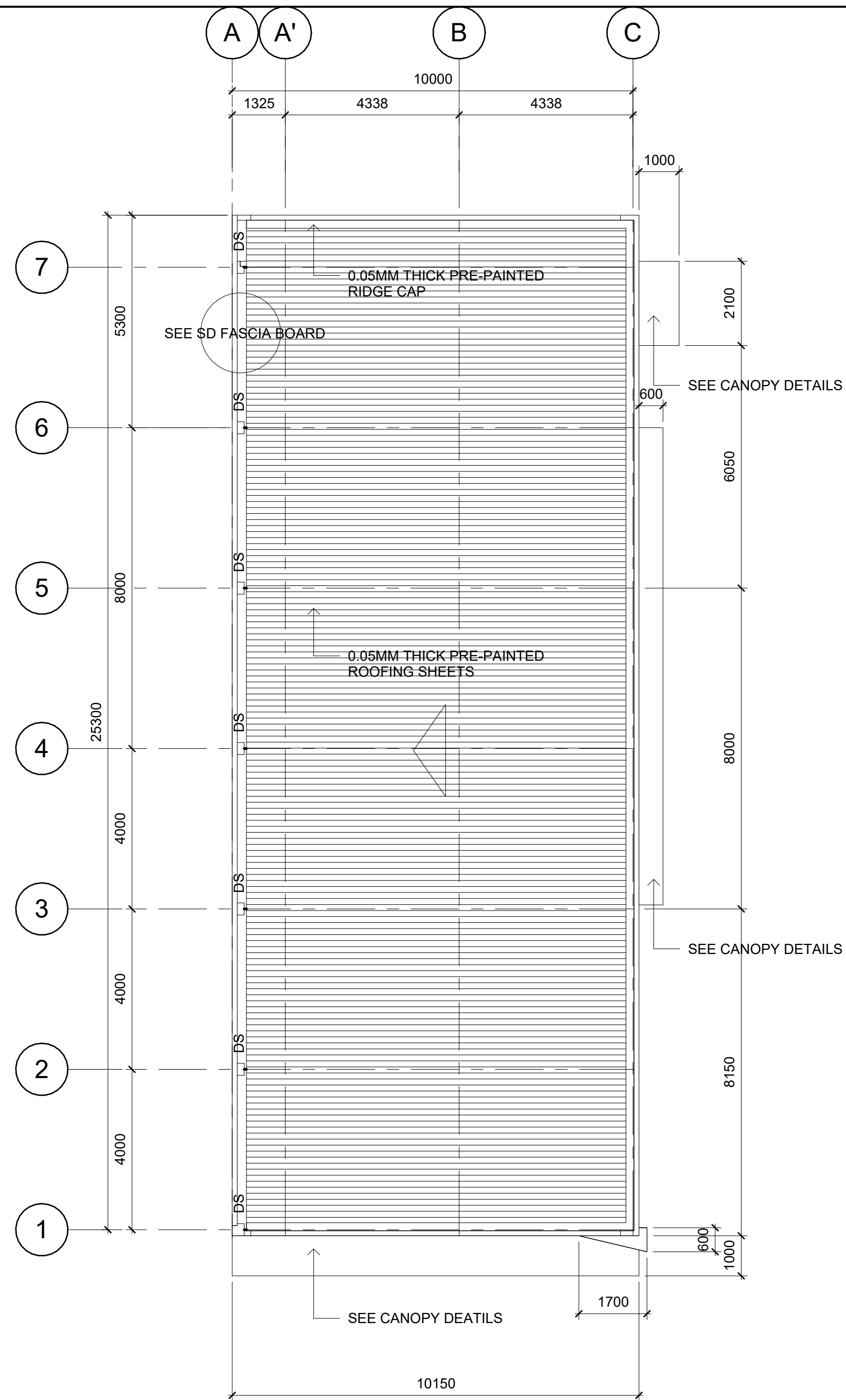
DETAIL OF STEEL TRUSSES

1 : 50

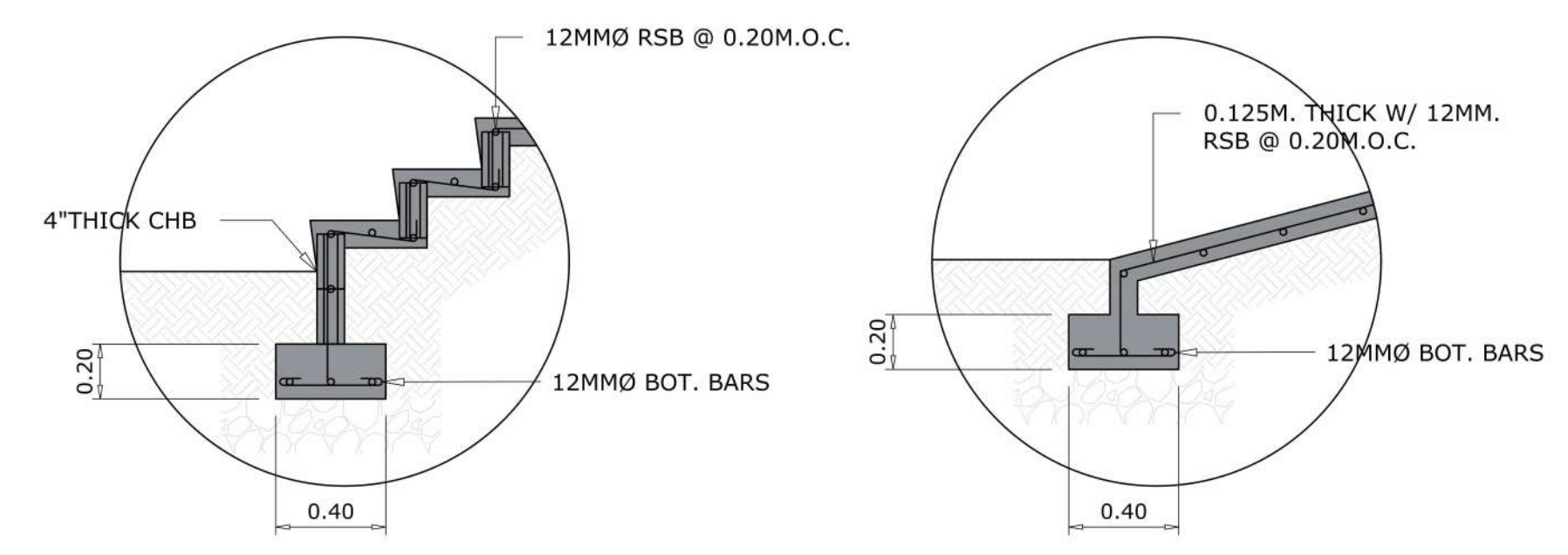
1 ROOF FRAMING PLAN

1 : 100

	TIN:	APPROVED BY:	PROJECT TITLE:	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT:	DESIGNED BY:	<table border="1"> <tr> <td>S6</td> <td>04</td> </tr> <tr> <td colspan="2">SHEET NO.</td> </tr> <tr> <td>11</td> <td>16</td> </tr> <tr> <td colspan="2">PLACE:</td> </tr> </table>	S6	04	SHEET NO.		11	16	PLACE:	
	S6	04															
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11	16																
PLACE:																	
PRC:			CONSTRUCTION OF VSU POWER PLANT BUILDING	ENGR. MARIO LILIO P. VALENZONA	DR. DANIEL LESLIE S. TAN	DR. EDGARDO E. TULIN	ROOF FRAMING PLAN SCHEDULE OF R.C. ROOF BEAM DETAIL OF STEEL TRUSSES DETAIL OF WALLS										
PTR:			LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A	DIRECTOR, PPO	VP OF ADMINISTRATIVE AND FINANCE	VSU PRESIDENT											
DATE:																	
PLACE:	STRUCTURAL ENGINEER																



1 DETAIL OF R.C. FASCIA BOARD 2 DETAIL OF R.C. RAMP UP

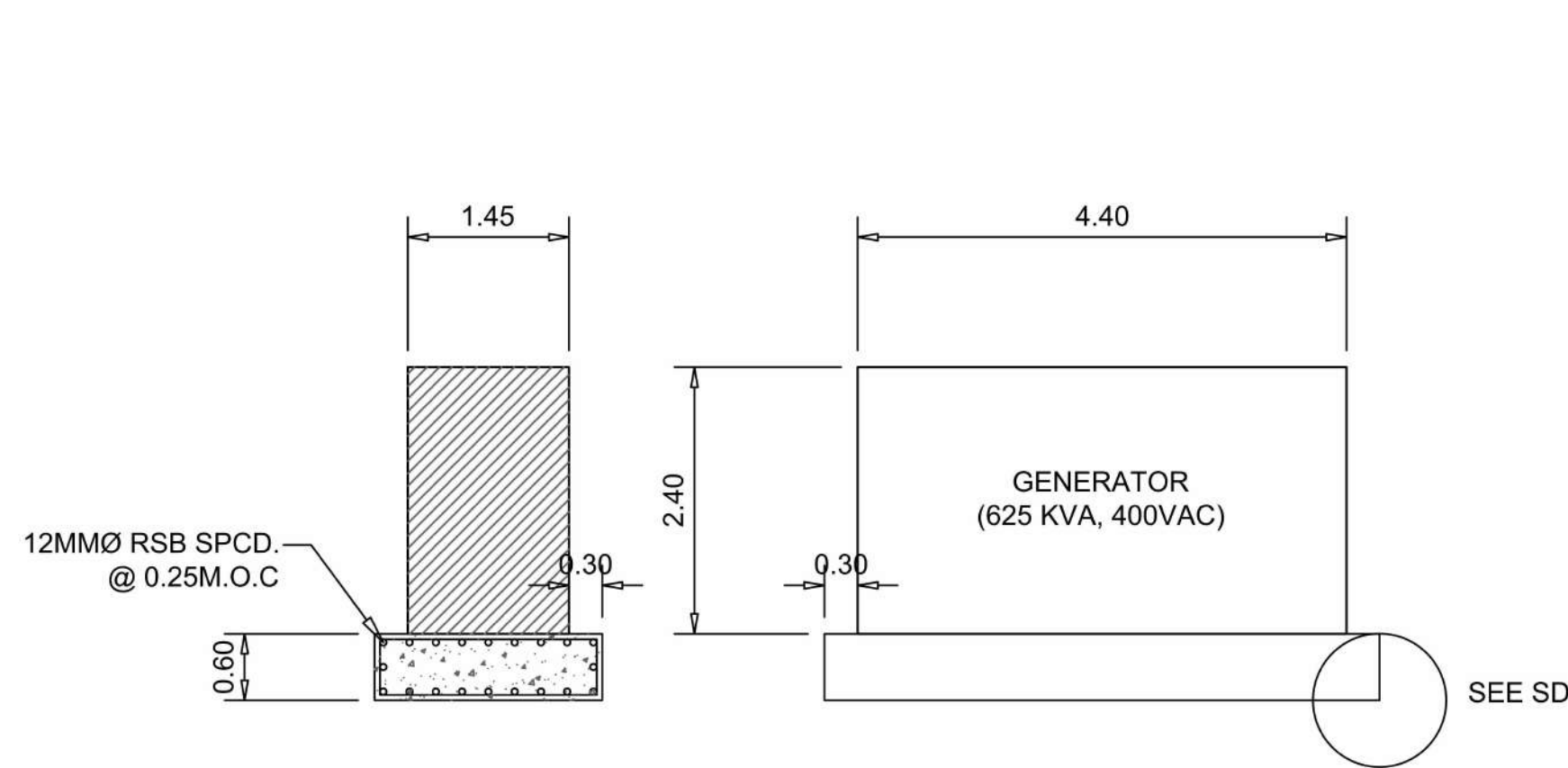


3 DETAIL OF R.C. STAIRS 4 DETAIL OF R.C. RAMP

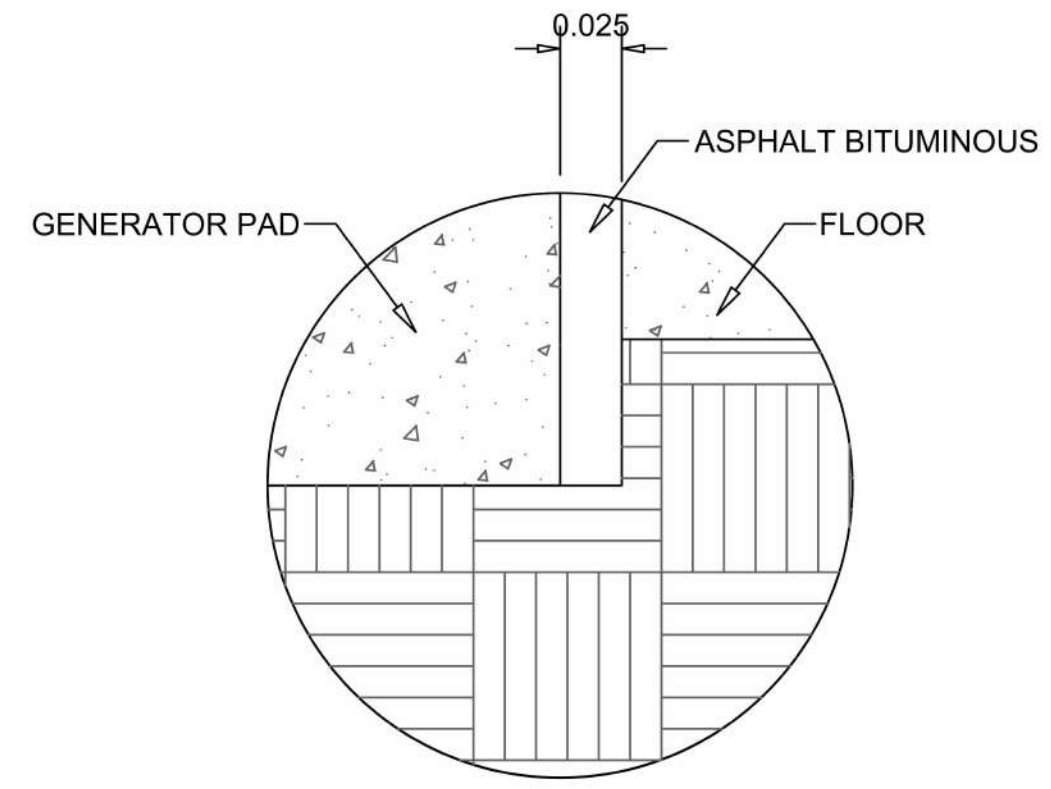
SPOT DETAILS
1 : 60

1 TOP OF ROOF PLAN
1 : 100

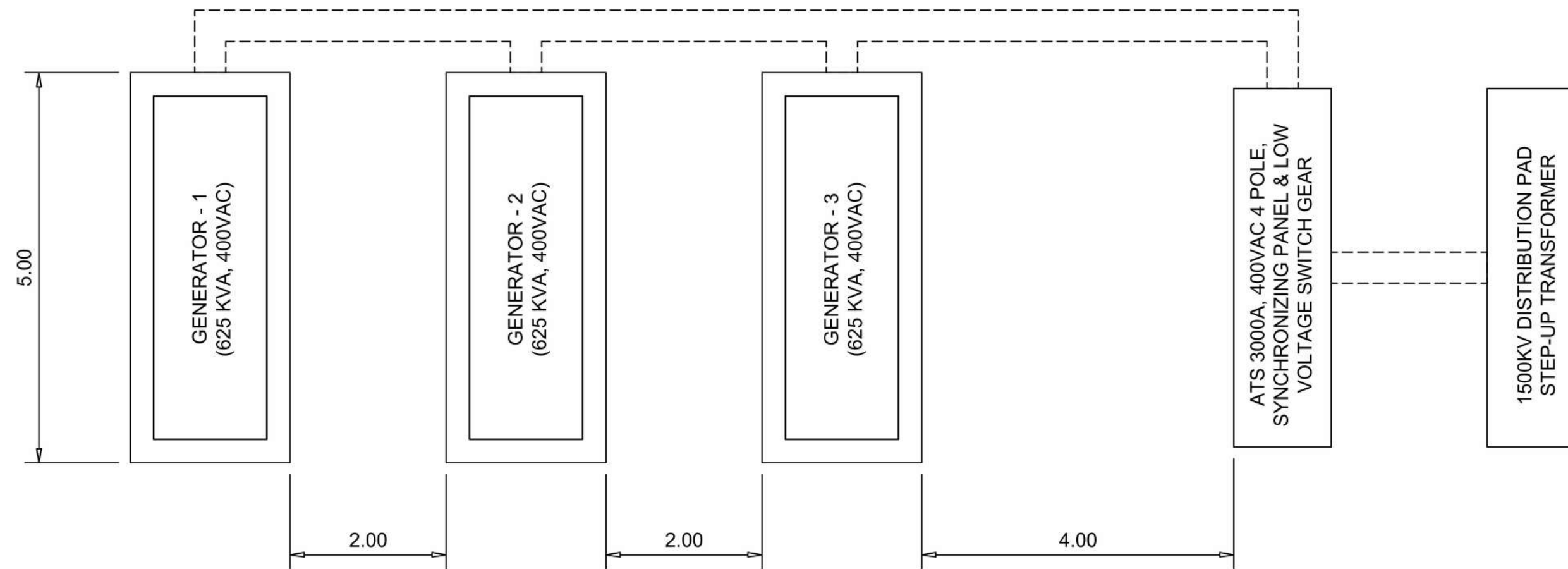
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PTR:		DIRECTOR, PPO	VP OF ADMINISTRATIVE AND FINANCE	VSU PRESIDENT	STARTED:	FINISHED:											
DATE:					PLACE:												



1 ELEVATION & SECTION



2 SPOT DETAIL



3 TOP VIEW

DETAIL OF GENERATOR PAD

1 : 50

	TIN:	APPROVED BY:	PROJECT TITLE: CONSTRUCTION OF VSU POWER PLANT BUILDING LOCATION: VSU MAIN CAMPUS, BAYBAY CITY, LEYTE PHILIPPINES, 6521 - A	CHECKED / APPROVED BY:	CONFORMED / APPROVED BY:	APPROVED BY:	SHEET CONTENT: DETAIL OF GENERATOR PAD	DESIGNED BY:	<table border="1"> <tr> <td>S6</td> <td>06</td> </tr> <tr> <td colspan="2">SHEET NO.</td> </tr> <tr> <td>13</td> <td>16</td> </tr> <tr> <td colspan="2">PLACE:</td> </tr> </table>	S6	06	SHEET NO.		13	16	PLACE:	
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